Hydrogeological Study
Allen Park Clay Mine
Allen Park, Michigan



**Professional Service Industries, Inc.**Michigan Testing Engineers Division

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November 24, 1981

Wayne Disposal, Inc. P.O. Box 5187 Dearborn, MI 48128

Attn:

Mr. Walter W. Tomyn, P.E.

Subject: Hydrogeological Study Allen Park Clay Mine Allen Park, Michigan MTE File No. 406-15046

#### Gentlemen:

Attached please find ten copies of our hydrogeological report for the Allen Park Clay Mine Landfill study located in Wayne County, Michigan.

This report has been prepared for inclusion with Ford Motor Company's application for a hazardous waste disposal facility operating license as required by the Hazardous Waste Management Act 1979, P.A. 64.

If you should have any questions regarding this project, feel free to contact us.

Very truly yours,

MICHIGAN TESTING ENGINEERS, INC.

Marshall P. Austin, P.E. Project Engineer

Randall DeRuiter Branch Manager

Stephen C. Schleede, P.E. Chief Soils Engineer

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HYDROGEOLOGICAL STUDY
ALLEN PARK CLAY MINE
ALLEN PARK, MICHIGAN

WAYNE DISPOSAL, INC.
P.O. Box 5187

DEARBORN, MICHIGAN 48128

NOVEMBER 24, 1981

BY

MICHIGAN TESTING ENGINEERS, INC.

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#### INTRODUCTION

## General

This report presents the results and recommendations regarding the hydrogeological conditions at the Allen Park Clay Mine landfill site located in Wayne County, within the city limits of Allen Park, Michigan. The report has been prepared as required by the Michigan Department of Natural Resources in compliance with the Hazardous Waste Management Act 1979, P.A. 64 for inclusion with the application for hazardous waste disposal facility operating license.

Authorization to perform this exploration and analysis was in the form of a letter agreement dated July 29, 1981 from Wayne Disposal, Inc. (operators of Allen Park Clay Mine) to Michigan Testing Engineers, Inc.

## Purpose

The purpose of the hydrogeological study was to determine the existing hydrogeological conditions; evaluate the potential for degradation of the ground and surface waters; and provide the basis for an effective and acceptable monitoring program.

## Scope

The scope of work for our services regarding this project included:

- 1. Installation of five monitoring wells into the water bearing aquifer at or near the hardpan soils approximately 75 to 85 feet below the existing grade. The wells are to be used for sampling the groundwater and for studying the groundwater flow regime.
- 2. Securing soil samples (split spoon sampling in sands, shelby tubes in clays) during the well installation at intervals of; ten feet to the proposed trench bottom, every five feet for the next fifteen feet; then every 10 feet to the bottom of the boring. Laboratory tests performed on selected samples included: permeability determination, moisture contents, grain-size analysis (sieve and hydrometer) and Atterberg limits determination.

- 3. Obtain background information including: previous site studies, deep oil, water and gas wells, and existing hydrological information.
- 4. Determine subsurface soils and bedrock, their distribution, thickness and characteristics from field investigations (both past and present), other deep well logs and available geologic literature.
- 5. Determine hydrogeological conditions at the project area from available domestic well logs, previous hydrogeological reports and from current hydrogeological information obtained during this study. Determine surface drainage features and subsurface ground water flow from monitoring/observation wells and establish a subsurface ground water contour map.
  - Prepare a hydrogeological report including the above information and our recommendations for a ground water monitoring program.

## Previous Explorations

Prior to this report, Michigan Testing Engineers, Inc. has conducted other subsurface explorations at this site. The results of these investigations were presented under MTE File Numbers 64-8519, 64-9623, 406-05042 and 401-00115. Pertinent information from each of these reports has been appended. A brief summary of each report follows.

File #64-8519 - Drilling of ten borings to depths ranging from 55.5 to 81 feet below the existing ground surface. Observation/monitoring wells were installed in four of the ten borings made. Laboratory testing consisted of classification and permeability determination of selected soil samples and chemical analysis of selected water samples.

File #64-9623 - Permeability tests were conducted on remolded, on-site silty clay samples to determine their suitability as a landfill cover (capping) material.

<u>File #406-05042</u> - Installation of three, shallow groundwater observation wells.

 $\underline{\text{File } \#401-00115}$  - Permeability tests were conducted using remolded, on-site bulk clay material used for construction of the perimeter dike.

#### DESCRIPTION OF SITE

## Site Location

The Allen Park Clay Mine is bounded by Oakwood Boulevard, Interstate 94, Outer Drive, Snow Road and M-39 (Southfield Road). The site is located in Wayne County, within the city limits of Allen Park, Michigan (See Figures 1 and 2).

## Existing Conditions

The Allen Park Clay Mine began operations in 1956. The 233 acre site is owned and operated by Ford Motor Company and presently is accepting only waste material from Ford Motor Company operations. A small portion of these wastes are classified as hazardous. It is planned to obtain an operating license for acceptance of hazardous wastes for a 16½ acre parcel within the site boundary. The limits of this area are shown on Figure 3. The types of classified hazardous waste to be disposed of include: Emission Control dust/sludge from the primary production of steel in electric furnaces (U.A. EPA Hazardous Waste No. K061) and Decanter tank tar sludge from coking operations (Hazardous Waste No. K087). Currently, only Cell No. 1 within the proposed licensing limits is accepting classified hazardous waste. The design elevation of the bottom of this cell is approximately +550 (USGS), or about 45 feet below grade.

Construction of a clay dike around the perimeter of the site is presently near completion.

## <u>Site Drainage</u>

To facilitate removal of surface runoff at the site, a shallow ditch has been excavated partially around the outside of

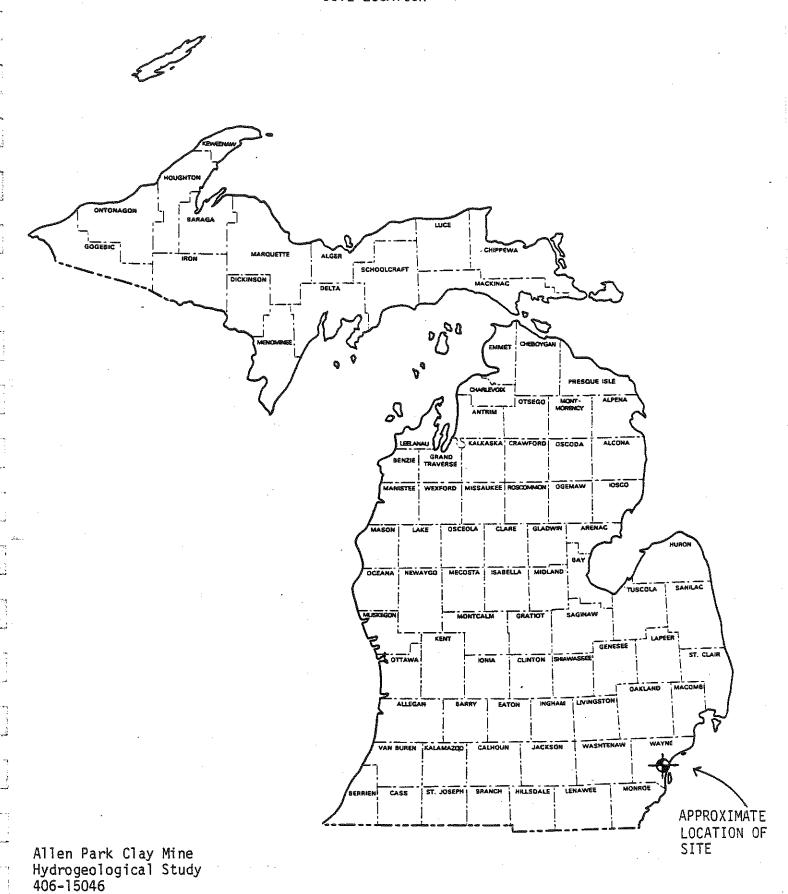
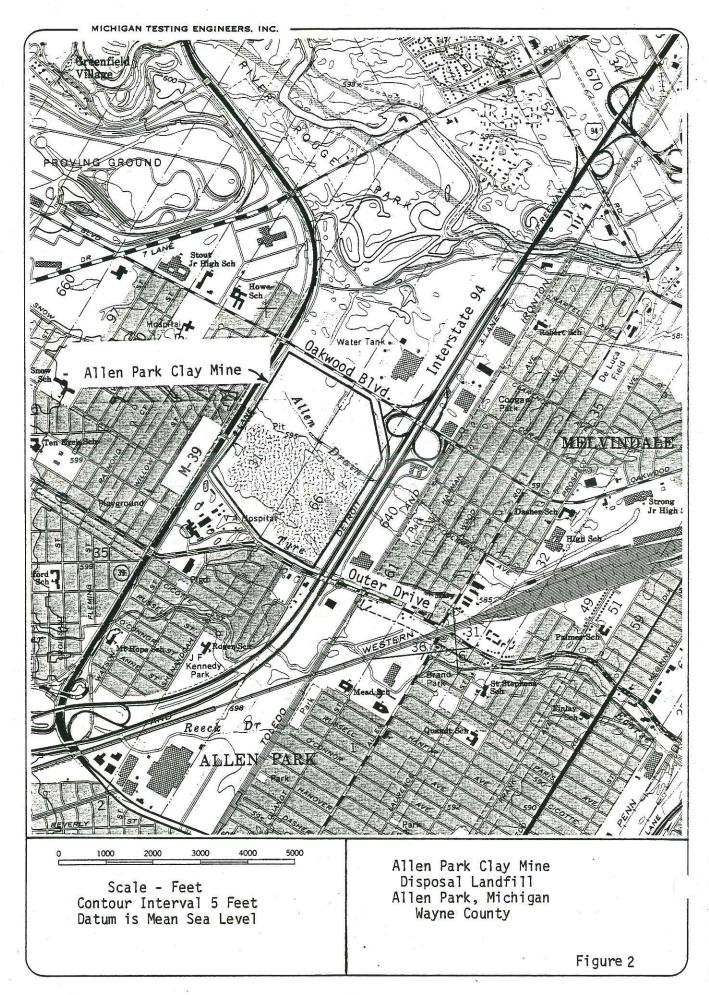


Figure 1



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the perimter dike beginning along the Southfield Freeway (M-39) traveling northeast to Oakwood outside the clay dike, then southeast along Oakwood around to I-94 where it empties into a temporary settling pond near Monitor Well W-103 before emptying into the Allen Drain, located approximately 250 feet southwest of Monitor Well W-103.

In addition, runoff from non-hazardous areas from within the facility are collected by ditches which empty into the perimeter ditch described above. A portion of the precipitation which collects in Cell No. 1 (where classified hazardous wastes are stored) will be used for dust control within this cell and any excess runoff will be hauled by tankers to the Ford Motor Company Rouge Plant. (Presently, Cell No. 2 contains no waste and runoff will be periodically pumped to the perimeter drainage ditch.)

The Tyre Drain collects runoff along Snow Road and Outer Drive as it travels to south to southeast direction towards I-94.

Both drains are enclosed as they travel southeast under I-94. The Tyre Drain empties into the Allen Drain which eventually empties into the Rouge River. A drainage map obtained from the office of the Wayne County Drain Commissioner is found on Figure 4.

From discussions with the Water Management Division of the Michigan Department of Natural Resources and others, it was learned that no portion of the site is contained within the limits of the 100 year flood plain of either the Rouge or Ecorse Rivers.

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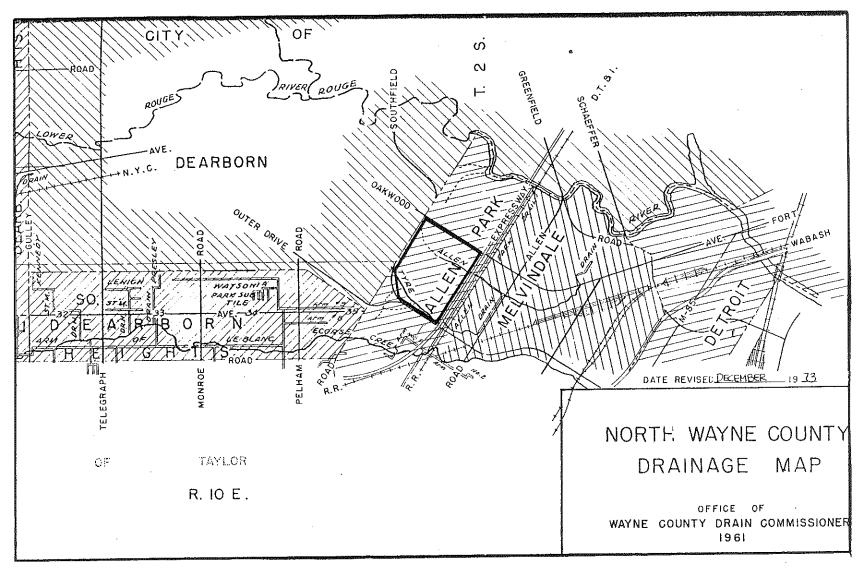


Figure 4

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#### FIELD EXPLORATION

## Scope

The field exploration to determine the engineering characteristics of the subsurface materials, included a reconnaissance of the project site, making the borings, performing standard penetration tests and recovering both split and undisturbed shelby tube samples.

Five soil test borings have been made, and these were drilled to depths ranging from 80 feet to 93 feet below the existing ground surface. They were drilled in the locations determined during a meeting with Wayne Disposal, Inc., the Michigan Department of Natural Resources and Michigan Testing Engineers, Inc.

The borings were located by means of normal taping procedures and are presumed to be accurate to within a few feet. Boring locations are shown on Figure 3. After completion of the borings, five PVC monitor/observation wells were installed in the bore holes. The installation procedure is described in a later section.

## Drilling and Sampling Procedures

The soil borings were performed with a drilling rig equipped with a rotary head. Conventional hollow-stem augers were used to advance the holes. Split spoon samples were obtained in conjunction with performing the standard penetration test in accordance with A.S.T.M. Procedure D-1586. Undisturbed samples were obtained using thin-walled sampling procedures in accordance with ASTM D-1587. Samples were identified according to boring number and depth and transported to the laboratory in special containers to protect against moisture loss.

## Field Tests and Measurements

Penetration Tests - During the split spoon sampling procedure,

standard penetration tests were performed to obtain the standard penetration tests were performed to obtain the standard penetration value of the soil. The standard penetration value (N) is defined as the number of blows of a 140 pound hammer falling 30 inches, required to advance the split-spoon sampler one foot into the soil. The sampler is lowered to the bottom of the drill hole, and the number of blows recorded for each of three successive increments of six inches penetration. (These three values are reported on the boring logs). The results of the standard penetration tests indicate the relative density and comparative consistency of the soils, and thereby provide a basis for estimating the relative strength and compressibility of the soil profile components.

Ground Surface Elevations - Elevations of the ground surface and monitor wells shown on the boring logs were determined by field engineers using conventional leveling techniques and is presumably accurate to within + 0.01 feet.

## Bench Marks

The elevation of the ground surface and top of monitor wells at W-101, W-102 and W-103 are referenced to the top of the railroad spike set in the northeast side of the Edison pole located approximately 200 feet west of the Oakwood entrance to the Allen Park Clay Mine on the south side of Oakwood Boulevard. The elevation of this bench mark was given as +596.80 (USGS).

Monitor wells and ground surface elevations at W-104 and W-105 are referenced to the bench mark established at the southwest corner of the top of the concrete lift station building between the east and west bound lanes of I-94 on the north side of Outer Drive.

The elevation of this bench mark was given at +591.24 (USGS).

Elevations of these bench marks were provided by others and their locations are shown on Figure 3.

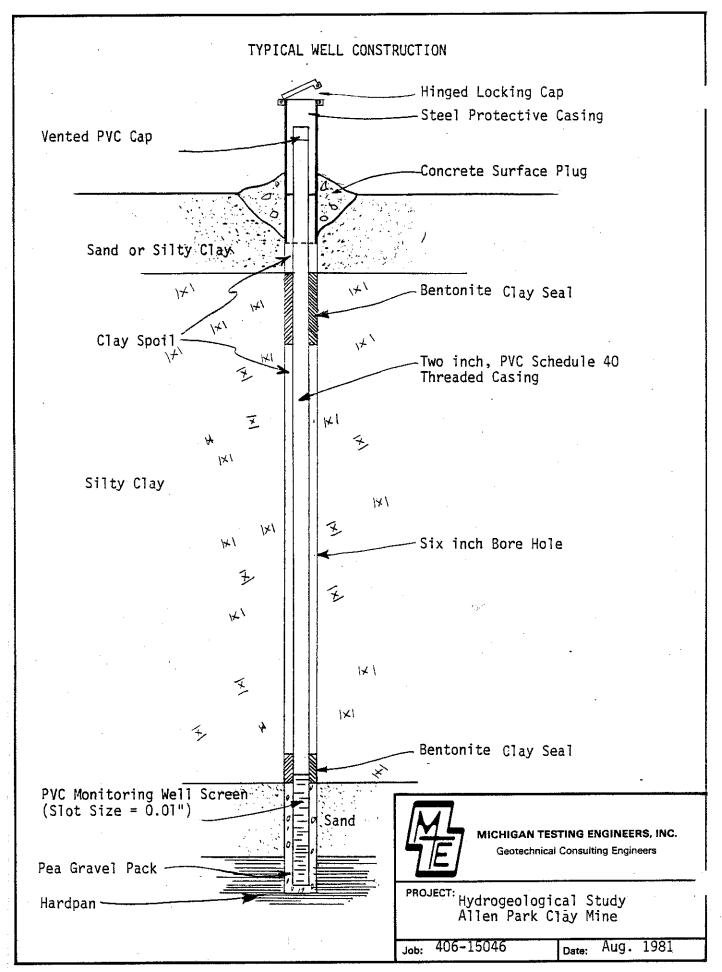
#### MONITORING/OBSERVATION WELL INSTALLATION PROCEDURE

The monitoring wells were constructed using 2-inch, threaded, schedule 40 PVC pipe. Upon completion of the soil boring, a five foot slotted PVC well screen (slot size - 0.01 inch) was attached to the bottom of the PVC standpipe and lowered to the bottom of the borehole within the sand and hardpan layers. A pea gravel pack was constructed around the entire length of the screen and a bentonite seal (approximately 2 feet thick) placed over the gravel pack within the borehole to prevent seepage from above.

The borehole was then backfilled with the clay spoil from the boring operation to the surface with a two-foot bentonite seal placed at a depth of approximately 15 feet. Five-inch diameter steel casing was placed over the standpipe and grouted in place to a depth of about four feet below grade to protect the PVC standpipe. Due to the artesian water conditions from the deep aquifer, the well and steel casing are approximately 10 feet above the existing grade. Locking caps were provided to reduce the potential for vandalism. A typical cross section of the well construction is shown on Figure 5.

Upon completion of the well installation, the wells were developed by flushing (with clean water) any soil fines or other foreign material from within the well casing. Subsequently, the well casing, ground surface and ground water elevations were determined at each well and are shown on Table 1. Upon development of Well No. W-102, it was discovered that the well was (and presently, still is) flowing and, therefore, to properly determine the ground water elevation, a piece of PVC casing (approximately 3 feet in length) must be coupled to the existing casing to permit the water level to stabilize. The extension

should be left in place approximately one week prior to obtaining a reading, to account for hydraulic lag.



## ALLEN PARK CLAY MINE

## MONITOR WELL - WATER LEVEL READINGS

Well Number	Ground Elevation, Ft.	Well Elevation(1) USGS	Ground Water(2) Elevation 11-4-81	Ground Water <sup>(3)</sup> Elevation 5-29-81	Ground Water <sup>(3)</sup> Elevation 3-26-81
2	595.1	600.76	600.67	600.44	600.21
5	595.7	605.92	605.09	604.62	604.49
7	594.1	597.35	591.01	593.23	594.14
10	593.4	603.03	601.81	601.93	601.56
W-101	593.9	601.47	601.21		
W-102	591.3	600.81	603.22 <sup>(4)</sup>		
W-103	593.9	605.06	603.52		
W-104	594.1	603.82	603.81		
W-105	594.5	604.08	603.86		

<sup>(1)</sup> Well Elevation is recorded as top of standpipe.

<sup>(2)</sup> Data Recorded by Michigan Testing Engineers, Inc.

<sup>(3)</sup> Data obtained from Michigan Department of Natural Resources.

<sup>(4)</sup> Well extended temporarily to obtain water level.

### LABORATORY TESTING PROGRAM

The laboratory testing program included supplementary visual classification, and water content determinations on all samples. Selected samples were subjected to dry unit weight determination, Atterberg limit tests (ASTM D-423 and D-424), grain size analyses (ASTM D-422) and permeability determination.

All phases of the laboratory testing program were conducted in general accordance with applicable ASTM Specifications, and the results of these tests are found on the accompanying boring logs and/or on Table 2 which follows.

Allen Park Clay Mine Allen Park, Michigan

## SOIL CLASSIFICATION SUMMARY

Boring <u>Number</u>	Sample Depth,Ft.	Moisture Content,%	Atter <u>LL</u>	berg Li	mits PI	Permeability cm/sec	USC* Classification
TOT-W	15	22.3	24	16	8	$3.8 \times 10^{-8}$	CL
	20	33.4	36	17	19	$3.5 \times 10^{-8}$	CL
-	30	19.3	27	16	11	•	CL
	35	19.3	29	15	14	$2.3 \times 10^{-8}$	CL
	40	16.6	21	14	7	-404	CL-ML
	45	20.1	30	16	14	2.1 x 10 <sup>-8</sup>	CL
	55	21.9	32	17	15	us	CL
	65	15.4	17	10	7	459	CL-ML
	75	18.5	27-	16	11		CL
	80	15.7		450		<b></b>	CL
W-102	5	15.6		webs	<b>sat</b>	-	-
	10	16.0	25	14	11	<b></b>	CL
	20	19.9	21	12	9	=	CL
	30	18.5	24	15	9	<b>.</b> .	CL
	35	14.6	20	13	7		CL-ML
	40	20.8	28	16	12	. 444	CL
	45	20.6	28	14	14	$2.0 \times 10^{-8}$	CL .
	55	21.1	33	16	17	was	CL
	65	40.4	47	21	26	$1.8 \times 10^{-8}$	CL
	80	24.2	35	20	15		CL
	85	11.6	19	12	7	. <b></b>	CL-ML

<sup>\*</sup>Indicates Unified Soil Classification

Sheet 1 of 3

Table 2

Allen Park Clay Mine Allen Park, Michigan

## SOIL CLASSIFICATION SUMMARY

Boring Number	Sample Depth,Ft.	Moisture Content,%	Atter LL	berg L PL	imits PI	Permeability cm/sec	USC* Classification
W-103	10	27.6	45	23	22		CL
	20	26.8	28	17	11	-	CL
	30	17.6	25	15	10	-	CL
	35	18.8	24	14	10	-	CL
	40	19.9	25	15	10	<b>~</b>	CL
	45	20.1	28	16	12	$2.4 \times 10^{-8}$	CL
	55 <sub>.</sub>	20.5	30	16	14	-	CL
	65	23.3	32	17	15	$3.0 \times 10^{-8}$	CL .
	75	22.3	28	17	11	<b>-ca</b>	CL.
	85	16.0	26	15	11	<b>-</b>	CL
W-104	5	26.3	-	-	-	-	-
	10	31.1	47	23	24	-	CL
	20	38.5	52	23	29	-	СН
	30	19.5	25	15	10	•	CL
	35	-19.5	26	15	11		CL·
	40	19.5	25	15	10	-	CL
	45	18.7	24	15	9	$2.3 \times 10^{-8}$	CL
	55	22.0	31	17	14	•	CL
	65	26.0	35	17	18	2.3 x 10 <sup>-8</sup>	CL
	75	8.8		-	-	<u>-</u>	SM
	80	19.0	-	-	<b>-</b>	-	CL
I							1

\*Indicates Unified Soil Classification

Sheet 2 of 3

Table 2

Allen Park Clay Mine Allen Park, Michigan

## SOIL CLASSIFICATION SUMMARY

Boring <u>Number</u>	Sample Depth,Ft.	Moisture Content,%	Atte	rberg Li <u>:P</u> L	imits PI	Permeability cm/sec	USC* Classification
W-105	10	29.8	40	- 21	1'9		CL
	20	33.9	38	19	19	wa-	CL
	30	20.1	25	15	10	<b></b> .	CL
	35	19.7	30°	15	15	-	CL
	40	20.4	25	16	9		CL
	45	20.8	30	16	14	$2.3 \times 10^{-8}$	CL
	55	21.5	31	15	16	400	CL
	65	25.6	34	20	14	$3.2 \times 10^{-8}$	· CL

Additional lab data from past studies is included in Appendices C, E and F.

Sheet 3 of 3

Table 2

<sup>\*</sup>Indicates Unified Soil Classification

#### GEOLOGY

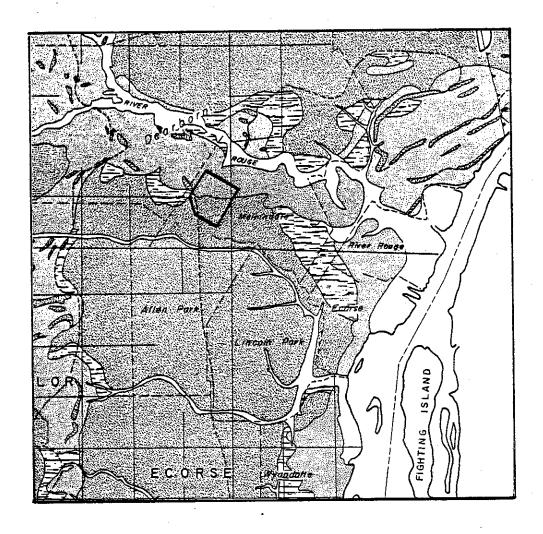
The present surface features of the area around southeastern Wayne County were formed during the Wisconsin stage of the Pleistocene glaciation. The Allen Park Clay Mine is situated within a glacial lake plain that was created when glacial meltwater became impounded between the moraines to the northwest and the receding Huron - Erie ice lobe to the southeast.

The surficial soils as shown on Figure 6 indicate deposits of either Lacustrine and Delta sand towards the northern half of the site and Lacustrine clay over the remainder. This, in general, was verified by the deposits of sand found at shallow depths at boring's W-101, W-102 and W-103 and the lack of a shallow sand deposit at W-104 and W-105.

The variation in composition and thickness of the glacial soils can be extreme for the glaciated area of southeast. Michigan. However, the soils encountered at the project site consists predominantly of silty clays with varying amounts of fine sand with occasional pebbles or fine gravel. The overburden material as shown on Figure 7 is generally anywhere from 80 to 100 feet thick as confirmed by soil borings and other available geological information.

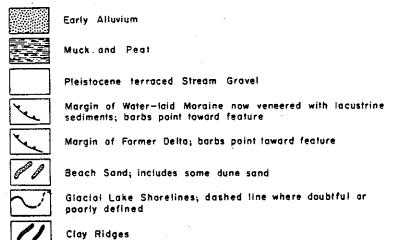
The elevation of the bedrock surface within this region is likely to be irregular due to erosion both prior to the glacial epoch, and from later repeated glacial advances. Figure 8 reveals a difference in bedrock elevation of up to 25 feet across the Allen Park site. The type of bedrock immediately below the site consists of Middle Devonian Dundee Limestone approximately 75 to 100 feet in thickness underlain by 250 to 350 feet of Detroit River Dolomite of the same geologic

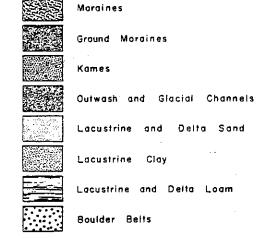
period (See Figure 9). For additional detailed information refer to the deep well logs in Appendix H obtained from the Michigan Department of Natural Resources. (Please note that the deep well logs were used only as a guide since no known deep wells exist within approximately 1 or 2 miles of the site.)

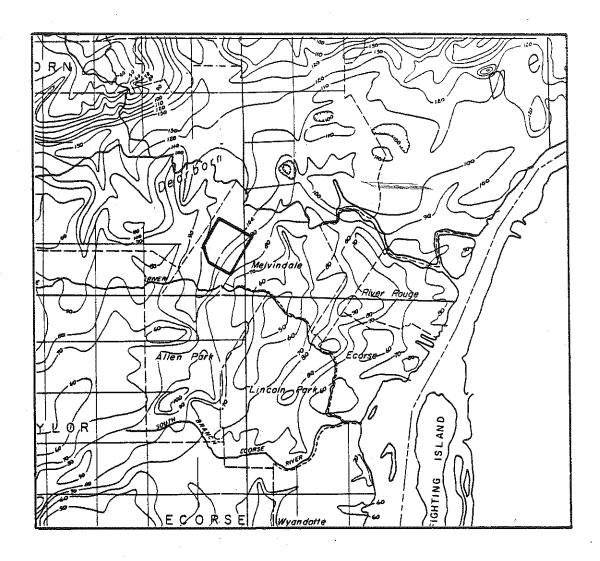


## GLACIAL FEATURES OF WAYNE COUNTY, MICHIGAN

#### LEGEND







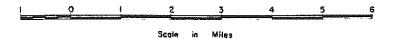
# GLACIAL DRIFT THICKNESS MAP OF WAYNE COUNTY, MICHIGAN

by

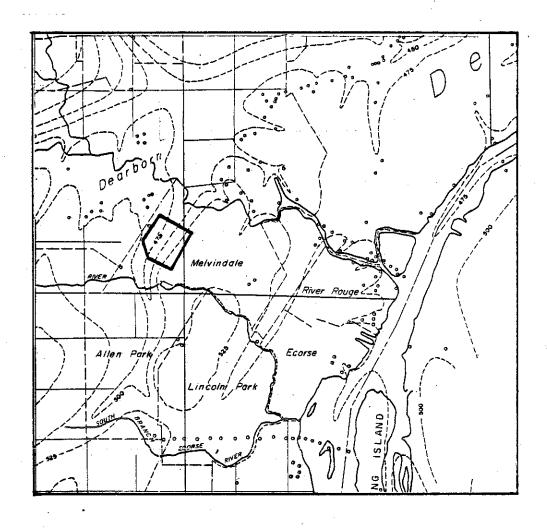
ANDREW J. MOZOLA

and

EUGENE I. SMITH
Wayne: State University-1967

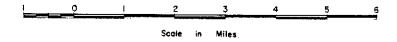


CONTOUR INTERVAL - 10 FEET



## TOPOGRAPHY OF THE BEDROCK SURFACE OF WAYNE COUNTY, MICHIGAN

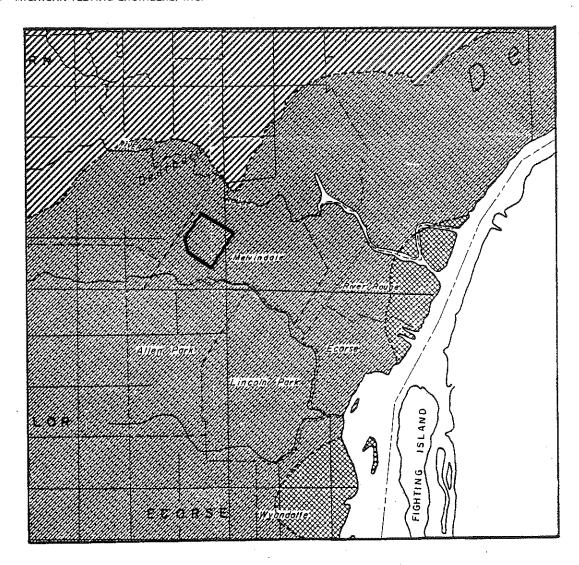
ANDREW J. MOZOLA Wayne State University-1967



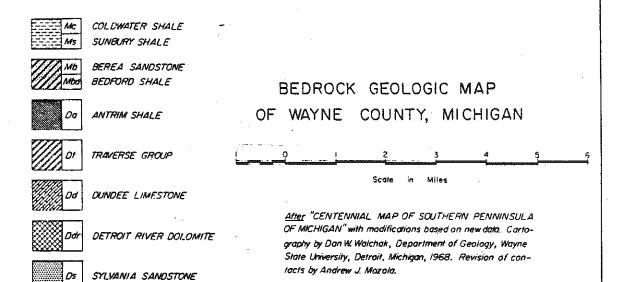
CONTOUR INTERVAL - 25 FEET

#### NOTES

- O TEST BORINGS AND WELLS REACHING OR PENETRATING BEDROCK:
- TEST BORINGS AND WELLS NOT REACHING BEDROCK
- BEDROCK ELEVATIONS FROM SEISMIC DATA.



# LEGEND



### SUBSURFACE CONDITIONS

## General

The type of foundation materials encountered have been visually classified and are described in detail on the boring logs. The results of the field penetration tests, water level observations, laboratory water content, Atterberg limits and unit weight determinations are presented on the boring logs. Representative samples of the soils are now stored in the laboratory for further analysis if desired. Unless notified to the contrary, all samples will be disposed of after three months.

The stratification of the soils as described herein, and as shown on the boring logs represents the soil conditions in the actual boring locations; other variations may occur between the borings. Lines of demarcation represent the approximate boundary between the soil types, but the transition may be gradual.

It is to be noted that, while the test borings are drilled and sampled by experienced drillers, it is sometimes difficult to record changes in stratification within narrow limits especially at great depths. In the absence of foreign substances, it is also difficult to distinguish between discolored soils and clean soil fill.

# Description of Foundation Materials

The following discussion is based upon subsurface information obtained from the soil boring data gathered from current and past geotechnical studies by Michigan Testing Engineers, Inc. at the Allen Park Clay Mine.

The soils encountered within the upper 10 to 15 feet at the boring (well) locations consist of either a fine to medium brown sand,

a brown to gray silty clay or a mixture of these two soils types. The cohesionless soil types are generally loose to medium dense in consistency while the more cohesive soils are medium to stiff. Below these soils, a layer of gray silty clay was encountered and this stratum continues to a depth ranging from about 72 feet to 86 feet below the existing ground surface. This silty clay stratum contains, in general, 15 to 25 percent sand (4.75 mm - 0.074 mm) and usually not more than five percent fine gravel (>4.76 mm). The natural water content of this layer ranges from about 15 to 40 percent, with the average near 20 percent. Results of Atterberg limits determinations reveal liquid limits (LL) and plastic indices (PI) ranging from 17 to 52 percent and 7 to 29 percent, respectively. The majority of the samples, however, showed an average LL of about 28 and a PI of about 12. Classification of this material by the Unified Classification System indicates this soil to be predominantly a (CL) soil (Inorganic clay of low to medium plasticity). Within this strata, occasional zones of borderline (ML-CL) soils are present and one test confirmed a zone of (CH) soil. (A description of the Unified Classification System is found on Figure 10).

Eleven permeability tests were performed on selected shelby tube samples obtained from the silty clay stratum during the current field investigation. Results of these tests indicate the coefficient of permeability ranges from  $1.8 \times 10^{-8}$  cm/sec to  $4.1 \times 10^{-8}$  cm/sec. (Results of permeability tests from previous investigations are found in Appendices C, E and F.)

Below the silty clay, a layer of gray, saturated medium sand (SP) with a trace of fine gravel was encountered ranging from about one

TABLE 2.16 UNIFIED SOIL CLASSIFICATION SYSTEM. (ASTM D-2487)

Majo	or Division	75	Group Symbols	Typical Names		Laboratory Class	sification Criteria		
	ction is ze}	gravels no fines)	GW	Well-graded gravels, gravel-sand mix- tures, little or no fines	Determine percentages of sand and gravel from grain-size curve.  Depending on percentage of fines (fraction smaller than No. 200 sieve size), coarse-grained soils are classified as follows:  Less than 5 per cent  More than 12 per cent  Borderline cases requiring dual symbols <sup>b</sup> 5 to 12 per cent	$C_U = \frac{D_{60}}{D_{10}}$ greater than 4; $C_C$	$\frac{(D_{30})^2}{D_{10} \times D_{60}}$ between 1 and 3		
sieve size)	Gravels If of coarse fra No. 4 sieve si	Clean gravels (Little or no fines)	G <b>P</b>	Poorly graded gravels, gravel-sand mix- tures, little or no fines	rve. 200 șieve size), co W, SP M, SC cases requiring du	Not meeting all gradation re	quirements for GW		
than No. 200	Gravels (More than half of coarse fraction is larger than No. 4 sieve size)	vith fines le amount nes)	GM <sup>a</sup> u	Silty gravels, gravel-sand-silt mixtures	rain-size curve. er than No. 200 și GW, GP, SW, SP GM, GC, SM, SC	Atterberg limits below "A" line or P.I. less than 4	Above "A" line with P. between 4 and 7 are borde line cases requiring use		
Coarse-grained soils naterial is larger than	(More	Gravels with fines (Appreciable amount of fines)	GC	Clayey gravels, gravel-sand-clay mix- tures	om grain-size cu maller than No. GW, GP, S GM, GC, S Borderline	Atterberg limits below "A" line with P.J. greater than 7	dual symbols		
Coarse-g material	.2		sw	Well-graded sands, gravelly sands, little or no fines	gravel fro	$C_U = \frac{D_{60}}{D_{10}}$ greater than 6; $C_C$			
Coarse-grained soils (More than half of material is larger than No. 200 sieve size)	barse fraction 4 sieve size)	Clean sands (Little or no fines)	SP	Poorty graded sands, gravelly sands, little or no fines	of sand and ge of fines ( lows:	Not meeting all gradation re	equirements for SW		
	Sands (More than half of coarse fraction smaller than No. 4 sieve size)	n fines semount	SMª u	Silty sands, sand-silt mixtures	Determine percentages of sand and gravel from grain-size curve.  Depending on percentage of fines (fraction smaller than No. 200 soils are classified as follows:  Less than 5 per cent  More than 12 per cent  5 to 12 per cent	Atterberg limits above "A" line or P.I. less than 4	Limits plotting in hatch zone with P.I. between and 7 are borderline car		
	(More tha	Sands with fines Appreciable smount	sc	Clayey sands, sand-clay mixtures	Determine percent Depending on perc soits are classified Less than 5 per of More than 12 per 5 to 12 per cent	Atterberg limits above "A" line with P.I. greater than 7			
		han 50)	ML	Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity	-	Plasticity Ch	art		
200 sieve	Sitts and clays	mit less t	CL.	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays					
Is then No.	N.S.	(Liquid limit less than 50)	OL	Organic silts and organic silty clays of low plasticity			СН		
Fine-grained soils material is smaller than No. 200 sievel		(Liquid limit greater than 50)	мн	Inorganic silts, micaceous or diatoma ceous fine sandy or silty soils, elastic silts	asticity inde	. File	OH and MH		
Fir alf mate	Silts and clavs	nit great	СН	Inorganic clays of high plasticity, fa clays		CL			
(More than half		(Liquid lin	ОН	Organic clays of medium to high plasticity, organic silts	0	O 10 20 30 40 50	60 70 80 90 100		
3	Highly	organic soils	Pt	Peat and other highly organic soil		Liquid l	imit		

<sup>&</sup>lt;sup>8</sup>Division of GM and SM groups into subdivisions of d and u are for roads and airfields only. Subdivision is based on Atterberg limits; suffix d used when L.L. is 28 or less and the P.I. is 6 or less; the suffix u used when L.L. is greater than 28.

<sup>8</sup>Borderline classifications, used for soils possessing characteristics of two groups, are designated by combinations of group symbols. For example: GW-GC, well-graded gravel-sand mixture with clay binder.

to six feet in thickness. This sand, in general, overlies an extremely dense layer of silty sand and clay (Hardpan). At boring (well) W-104 the sand layer, approximately 6 inches thick, is apparently sandwiched between two layers of the hardpan soils with the upper layer approximately 2½ feet thick. At W-101, the hardpan layer encountered was about one foot thick, underlain by a very stiff, very silty clay.

(Detailed soil boring logs are presented in Appendix A. A summary of laboratory test results was included previously on Table 2).

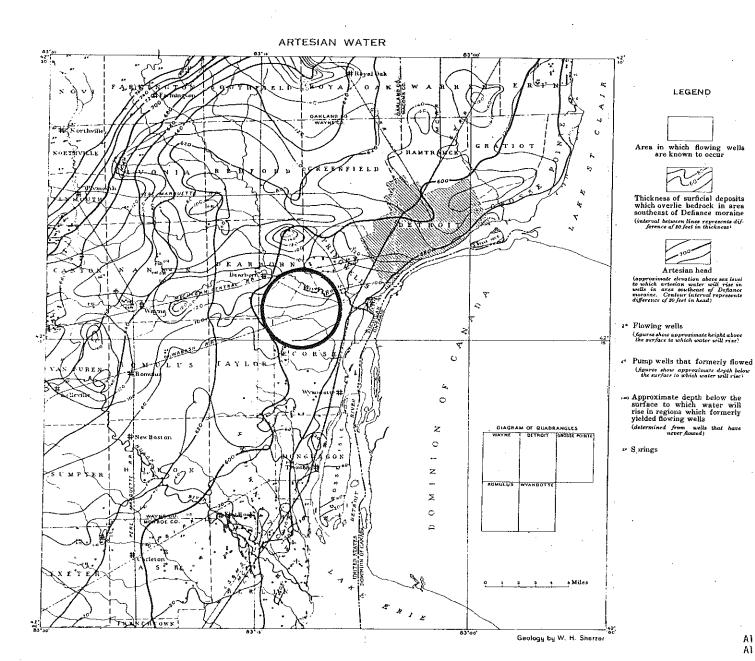
### GROUND WATER

## General

Except for a minor water table encountered within the shallow sand layer near the surface at W-102 and W-103, the only other ground water encountered during this investigation was within the sand and hardpan layers below the thick deposit of silty clay. The elevation at which this aquifer was encountered ranged from +505 (W-102) to +522 (W-105). This lower water bearing stratum is an artesian aquifer with static water levels reaching a height of up to 12 feet above the existing ground surface. From available literature, it is believed that this aquifer is recharged within the belt of moraines near west and northwest Wayne County. Figure 11 (taken from U.S.G.S. survey, circa: early 1900's) shows a contour map of artesian water in an area southeast of the Defiance Moraine. The artesian head is shown by lines of equal pressure. From this figure, the hydraulic gradient was calculated to be decreasing at a rate of approximately 3 to 4 feet per mile in an east-southeast direction. The static ground water elevations recorded at the monitor wells on November 4, 1981 are shown on the Ground Water Elevation Map in Figure 12. The recorded values show the general trend of groundwater eminating near Well No. 5 and flowing outward toward the other wells, and especially towards localized low areas near Well No. 7 and 2.

It is believed that the regional trend of ground water flow in this area is in the east-southeast direction, as shown on Figure 11, however, the water level readings shown on Figure 12 indicate ground water flow towards localized depressed areas near Well No's. 2 and 7. As more data becomes available, this condition can be verified.

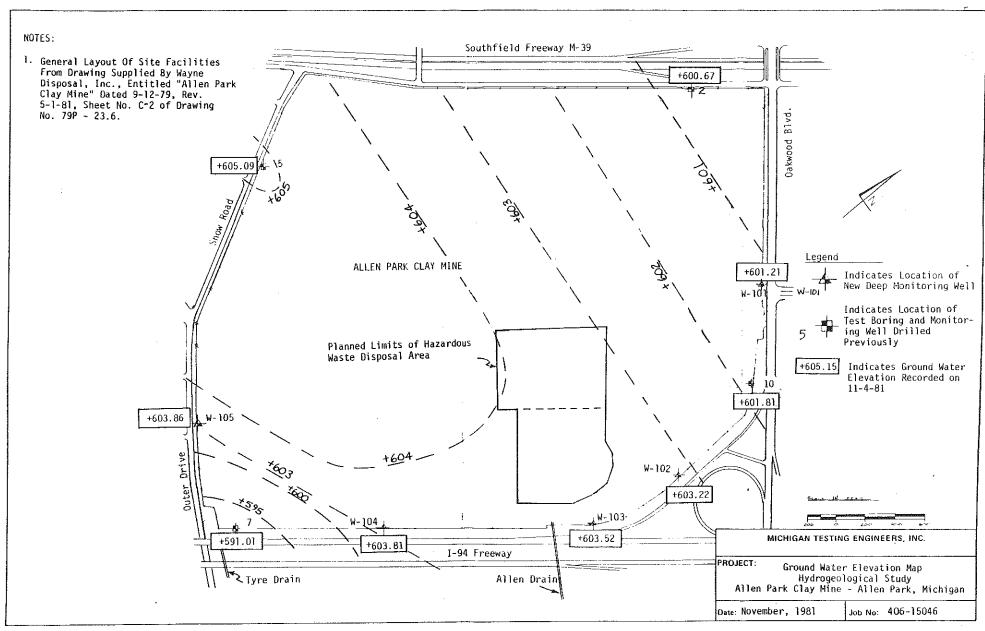
It should be noted that the static water levels recorded at



Allen Park Clay Mine Allen Park, Michigan

Fig. ..... 11

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the site monitor wells are very similar to those shown on the artesian water contour map which may indicate a fairly consistent hydraulic condition exists within this aquifer. Also, permeability can vary within a confined aquifer due to local changes within lithology and therefore the piezometric surface may not be a smooth plane, but rather somewhat irregular.

## <u>Ground Water Quality</u>

The latest chemical analyses of water samples obtained from the observation/monitoring wells at the Allen Park Clay Mine are shown on Table 3. Listed are all current wells, both within the deep and shallow aquifer, used to monitor both hazardous and non-hazardous landfilling operations. Listed on Table 3 (sheet 1 of 3) are the parameters for which the EPA has established interim drinking water standards. In general, the measured levels are at or below the established limits except for a few of the metals. At Well No. 2D (located within the deep aquifer), the measured level of cadmium was 0.24 mg/l; the EPA interim standard is 0.01 mg/l. It was suggested by Canton Analytical Laboratory (who performed the test) that the high cadmium level may be from the interior of the galvanized well standpipe. (It should be noted that cadmium levels at W-2 determined by the DNR during December, 1980 were below 0.020 mg/l.)

In addition, levels of lead at Well's W-2S, 5S and 10S ranged from 0.065 to 0.22 mg/l, which have exceeded the EPA interim standard of 0.05 mg/l.

Other parameters analyzed from monitor well samples are included on sheets 2 and 3 of Table 3. Currently, no established EPA standards are available for these parameters.

As additional ground water data becomes available, parameters which have exceeded recommended limits can be verified. (Refer to the ground water Monitoring Program section for the well sampling schedule and parameters to be analyzed).

# Useability of Aquifers

Shallow Aquifer - The Michigan Department of Public Health does not permit drinking water wells to be located less than 25 feet below the ground surface. Since the upper, shallow aquifer was found to be generally less than 10 feet below grade, this aquifer is not a useable source of drinking water.

<u>Deep Aquifer</u> - The lower aquifer is located approximately 70 feet below the existing grade at the Allen Park site. Chemical analyses of ground water samples show this source to water to be highly mineralized. In addition, a few of the parameters for which the EPA has drinking water standards have been exceeded. Therefore, to use the lower aquifer as a source of drinking water would probably require treatment.

From discussions with the Wayne County Department of Environmental Health and the City of Allen Park Water Board, it was learned that no known water wells within Allen Park exist and that both residential and commercial water service are supplied by the city of Detroit.

llen Park Clay Mine llen Park, Michigan

#### GROUND WATER QUALITY ANALYSIS (Values in mg/l unless otherwise noted)

#### WELL NUMBER

PARAMETER	<sub>W-2S</sub> (1)	W-20 <sup>(2)</sup>	W-5S	W-5D	W-7D	W-10S	W-10D	W-101D	W-102D	W-103D	W-104 D	W-105 D	LIMITS (Mg/1) (3)
Arsenic, As	_	ко.0003 (	4) _	-	-			-	КО.0003	K0.0003	K0.0003	- -	0.05
Barium, Ba	-	KO.1	-		-	-	<b>-</b> .	_	Ķ0.1	ко.1	KO.1	-	1.0
Cadmium, Cd	K0.02	0.24	K0.02	КО.02	K0.02	K0.02	K0.02	-	K0.01	KO.01	K0.01	<u>-</u>	0.01
Chromium, Cr	KO.05	ко.01	K0.05	KO.01	K0.01	K0.05	KO.01	ко.01	ко.01	ко.01	ко.01	K0.01	0.05
Flouride, F	-	0.94	-	-	-	-	-4	-	1.42	1.31	1.31		1.4 - 2.4
Lead, Pb	0.095	K0.05	0.22	ე.05	KO.05	0.065	KO.05	KO.05	КО.05	KO.05	K0.05	КО.05	0.05
Mercury, Hg	-	K0.0002	-	-	-	-	-	-	KO.0002	KO.0002	KO.0002	-	0.002
Nitrate as N, NO <sub>3</sub>	0.02	KO.1	0.04	KO.1	KO.1	0.15	KO.1	K0.1	K0.1	KO.1	K0.1	K0.1	10
Selenium, Se	-	КО.0003	-		-		<del>-</del>	-	K0.0003	K0.0003	КО.0003	_	0.01
S lver, Ag	_	K0.01	-	-	_	-	i <del>c</del>	-	K0.01	ко.01	K0.01	-	0.05
Fririn	_	K0.0002		-				-	K0.0002	KO.0002	K0.0002	-	0.0002
Lindane	-	K0.004	-	-	_	-	-	-	КО.004	КО.004	KO.004	-	0.004
Methoxychlor	-	K0.01	-	-	-		_	-	ко.01	ко.01	K0.01	_	0.1
Sa Toxaphene	-	ко.005	-	-	-	-	-	-	KO.005	КО.005	KO.005	-	0.005
2, 4-D		КО.1	_	-	<del> </del>	_	-	-	K0.1	K0.1	K0.1	-	0.1
2, 4, 5 - TP Silvex	-	ко.001	-	٠	-		**	~	KO.001	ко.001	KO.001	-	0.01
Radium, Ra	-	K5	-	-	_		-	-	K5	K5	K5	-	5
Gross Alpha pCi/l	_	K5	-	-	-	-	-	-	К5	к5	К5	-	15 pCi/l
Gross Beta pCi/l	-	K5	-	-	_	-		-	К5	К5	K5	_	4 Millirem/yr
Coliform Bacteria /100 ml	-	2	-	-	-			-	4	K2	4	-	1/100 ml

<sup>(1)</sup> W-2S: Indicates Well No. 2 located within the Shallow Aquifer.

#### Notes:

- I. Parameters listed in this Table represent a composite of the latest results of samples tested on one of the following dates:
  A.- December 1980 by: Michigan Department of Natural Resources
  B.- October 1981 by: Canton Analytical Laboratory
- II. Additional water quality data from previous studies is found in Appendix F.
- III. Well No's W-101 through W-105 were constructed using PVC pipe while the remainder were constructed using galvanized pipe. Sheet 1 of 3

 $<sup>(2)</sup>_{W-2D}$ : Indicates Well No. 2 located with the Deep Aquifer.

<sup>(3) &</sup>quot;EPA Interim Primary Drinking Water Standards", Federal Register, Appendix III, Part 265, p. 33257, May 19, 1980.

<sup>(4)</sup> K: Indicates "less than" value shown.

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# GROUND WATER QUALITY ANALYSIS (Values in mg/1 unless otherwise noted)

## WELL NUMBER

PARAMETER	W-2S	W-2D	W-5S	W-5D	W-7D	W-10S	W-10D	W-1010	W-102D	W-103D	W-104D	W-105D	LIMITS
Alkalinity .	157	164	255	640	92	127	184	_	-	-			Currently, the EPA has not established water quality limits for these para-
Ammonia NH <sub>3</sub>	0.48	0.5	0.12	0.3	0.75	0.42	0.5	0.5	0.5	0.40	0.5	0.40	meters.
BiCarbonate, HCU3	192	200	315	675	0	155	225	_	-	-	-	<b></b> .	
Calcium, Ca	300	200	340	10	370	280	370	190	160	37	310	330	
Carbonate, CO <sub>3</sub>	0	0	0	53	22	0	0	_	-	-	-	-	
Chloride, Cl	91	150	1200	126	150	180	150	135	130	25	140	145	
COD	15	2.6	84	3.8	1.9	20	7.0	1.6	0.6	1.6	1.0	1.0	
Copper, Cu	0.19	K0.02	0.04	0.2	КО.02	0.13	0.12	-	-	-	-	· -	
Cynaide, CN	ко.005	K0.005	K0.005	KO.005	0.019	ко.005	KQ.005	-	_	-	-	-	
Hardness	-	1600	-	800	950	-	1650	1050	1750	160	1500	1500	
Iron, Fe	3.7	КО.ОЗ	4.6	K0.03	KO.03	8.0	0.24	КО.03	КО.ОЗ	K0.03	0.32	1.4	
Magnesium, Mg	100	160	100	160	· 24	90	200	140	210	8.2	180	150	
Manganese, Mn	-	K0.01	-	-	_	-	-	-	ко.о1	КО.01	0.06	_	
Nickel, Ni	K0.05	0.05	КО.05	KO.05	КО.05	KO.05	K0.05	-	-	-	-	-	
Nitrite, NO <sub>2</sub>	K0.01	0.002	КО.01	KO.002	КО.002	0.01	K0.002	0.004	0.002	K0.002	KO.002	КО.002	
Nitrogen (Organic)	0.50	0.60	0.42	0.29	0.42	0.54	0.26	-	-	-	-	-	
PCB A-1242	КО.0001	K0.0001	ко.0001	KO.0001	K0.0001	K0.0001	KO.0001	-	-	-	-	-	
PCB A-1254	ко.0001	ко.0001	ко.0001	KO.0001	KO.0001	K0.0001	KO.0001	-		-	-	-	
PCB A-1260	K0.0001	ко.0001	ко.0001	K0.0001	K0.0001	KO.0001	ко.0001	~	-		-	-	
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See Sneet 1 of 3 for Notes.

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# GROUND WATER QUALITY ANALYSIS (Values in mg/l unless otherwise noted)

WELL NUMBER

	7		<del></del>	<del>.  </del>	<del></del>	WEI	L NUMBER					
PARAMETER	W-2S	W-2D	W-5S	W-5D	W-7D	W-10S	W-10D	W-101D	W-102D	W-103D	W-104D	W-105D
рН	7.8	7.7	7.2	9.6	10.0	7.6	7.8	7.1	8.4	8.6	8.0	7.1
рН	-	8.0	-	_	-	_	-	-	8.1	8.3	8.0	_
Н	-	7.9	-	-	<del> </del>	-	+	-	8.1	8.4	8.0	<del> </del>
На	-	.8.0	-	-		_	_	_	8.1	8.5	8.1	
Phenol	0.029	0.008	K0.005	0.021	0.023	0.005	0.009		K0.005	K0.005	K0.005	<u></u>
Potassium, K	3.3	5.3	1.5	4.2	19	2.5	4.9	_	_	_	10.003	_
Resistivity - TF	1900	1800	3100	1000	1400	1700	2900		<del> </del>	<del> </del>	<u> </u>	<u> </u>
Sodium, Na	100	110	380	100	120	95	90	120	100	6.0	100	90
Specific Conductivity, MMhos/cm	2200	2500	4100	1550	2250	2150	3000	2400	2500	300	2550	<del> </del>
specific Conductivity, uMhos/cm	i	2400	-	_			-		2450	300	2500	2600
pecific Conductivity, MMhos/cm	-	2200		_	<del> </del>		_		2300	300	2400	-
Specific Conductivity, MMhos/cm	1	2200	-	_	_	_	_	_	2300	300	2400	-
oulfate, SO <sub>4</sub>	1000	1050	200	240	1300	830	2100	1250	1200	46		7000
ulfide, S	K0.02	2.6	K0.02	0.28	0.18	K0.02	0.71		-	40	1350	1300
otal Organic Carbon	5.8	7.7	24	9	7	6.1	7	11	5.6	5.6	-	
otal Organic Carbon	~	7.0		-	, -	_	_	_ ''	6.0		6.0	11
otal Organic Carbon	-	7.7							5.6	6.0	6.8	-
otal Organic Carbon	-	7.6	_	_	_		_	_		5.6	6.6	
otal Organic Halides	-	0.005							6.6	6.6	6.6	-
otal Organic Halides	~	KO.005	_	_	ļ	~	-	-	0.0083	0.0308	KO.005	-
otal Organic Halides		KO.005			<del>-</del> [			-	0.0063	0.0302	K0.005	-
otal Organic Halides		1	-	-	-	-	-	-	0.0073	0.0311	K0.005	-
	- <sub>U</sub> (1)	K0.005							0.00886	0.0290	K0.005	-
_	-	Ü	*	U	U	U	U	- '	-	-	-	
inc, Zn	9.5	26	14	15	0.065	10 .	37	-	-	-	- [	

<sup>(1)</sup> Indicates Parameter was Undetected

\* Unknown Non-Halogenated Hydrocarbon

LIMITS (Mg/1)

Currently, the EPA has not established water quality limits for these parameters.

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## SUMMARY CONCLUSIONS AND RECOMMENDATIONS

## General

The results of this study indicate the geologic and ground water conditions at the Allen Park Clay Mine will permit the continued safe storage of the classified hazardous wastes. Soil borings made during the present and past studies reveal a thick, uniform silty clay layer (classified as predominantly a CL soil) which extends downward to an elevation ranging from +505 to +522 at the boring locations. Currently, the base elevation of Cell No. 1 is approximately +550. Therefore, the thickness of the clay layer below the cell is generally in excess of 25 feet. Laboratory permeability test results of undisturbed shelby tube samples of this silty clay layer from past and current studies at this site did not exceed  $6.0 \times 10^{-8}$  cm/sec.

In addition, no apparent continuous or extensive sand layers are present within this silty clay layer as confirmed during classification of secured soil samples.

Below the silty clay layer exists a lower, confined artesian aquifer with a static water level that, in general, rises to an elevation of +591 to +605 at the monitor wells. It is extremely unlikely that any leachate from the landfill will reach this aquifer, since there is a positive upward flow from the aquifer. The combination of the uniform, thick low permeable clays and the upward flow of ground water has created highly favorable conditions for containment of the hazardous wastes.

Ground Water Monitoring Program

Well No. 2 has been designated as the upgradient well and well number's W-102, W-103 and W-104 will be used as the downgradient wells for purposes of monitoring the ground water as required for the hazardous landfilling area. Additionally, it is recommended that Well No. 5 also be

used as an upgradient well since the static water level at this location within the lower aquifer had the highest recorded elevation of the deep monitoring wells.

It is recommended that the proposed RCRA rules regarding groundwater monitoring for hazardous waste facilities published in the May 19, 1980 Federal Register (Parts 265.91 through 265.94, pp 33240-42) be used as guidelines for monitoring the groundwater at Well No's. W-2, W-5, W-102, W-103, and W-104 at the Allen Park Clay Mine. The parameters and sampling schedule as outlined in this document follows:

1) Parameters for which the EPA has drinking water standards:

Arsenic Endrin
Barium Lindane
Cadmium Methoxychlor
Chromium Toxaphene
Floride 2, 4-D

Lead 2, 4, 5-TP Silvex Mercury Radium

Nitrate as N Gross Alpha Selenium Gross Beta Silver Coliform Bacteria

Concentrations of the above parameters should be tested on a quarterly basis for the first year.

II) Water Quality Parameters:

Chloride Phenols
Iron Sodium
Manganese Sulfate

III) Contamination Indicators: (At least four replicate measurements must be obtained for each sample).

pH Total Organic Carbon Specific Conductance Total Organic Halogen

After the first year, ground water quality parameters (Part II) must be analyzed at least annually and the Contamination Indicators (Part III must be analyzed at least semi-annually.

Ground water elevations should be determined at each monitoring well when sampling for chemical analysis. (Depending upon the climatic conditions and seasonal fluctuations within the aquifer, one or more of the monitor wells may be flowing. Therefore, to obtain a true ground water level, the flowing well must be extended with a coupled section of PVC pipe to allow the water level to stabilize. The well should be allowed to stabilize for at least one week to account for hydraulic lag prior to monitoring.)

The remainder of the monitoring wells should be sampled and analyzed for the standard parameters as required by the Michigan Department of Natural Resources for Type II Landfills.

Additionally, it is recommended that samples of the hazardous waste leachate be analyzed for the same parameters as outlined above for ground water samples for purposes of comparison.

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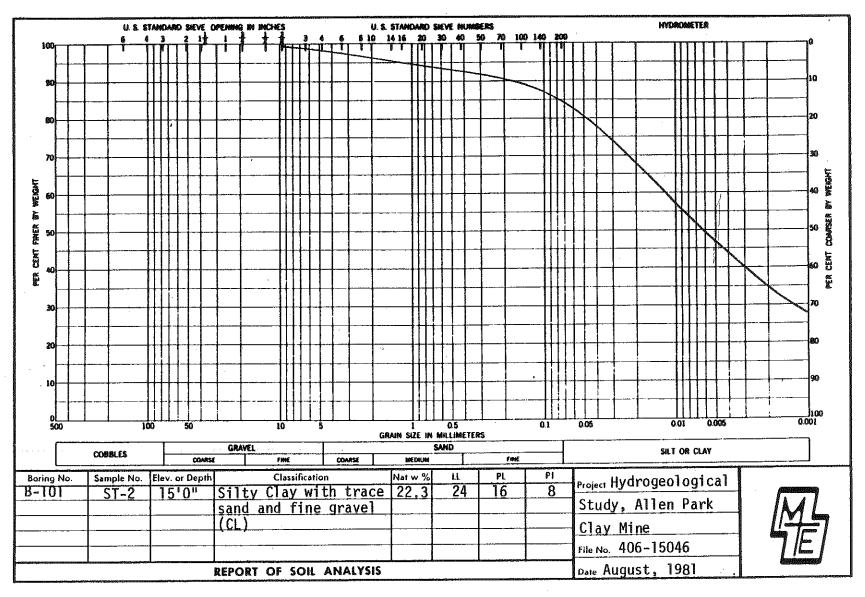
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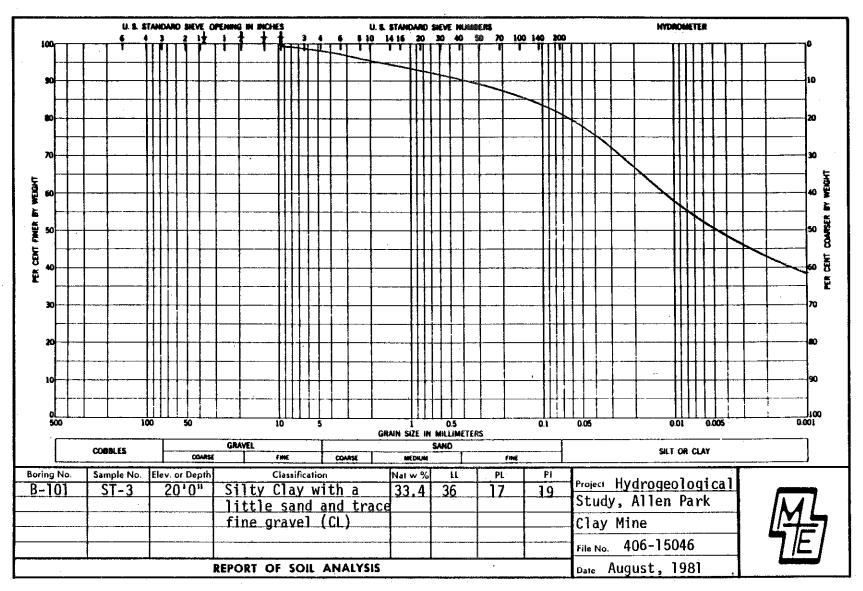
MICHIGAN TESTING ENGINEERS, INC. -

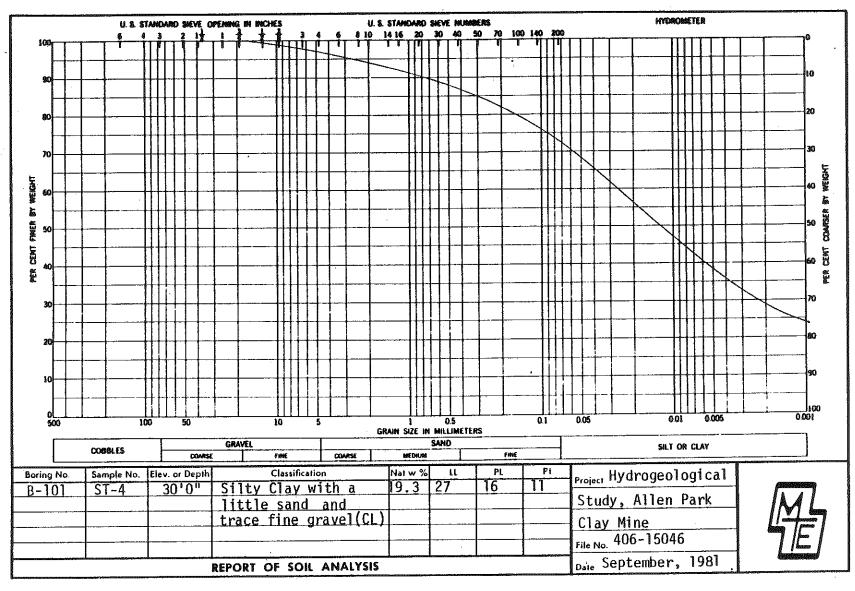
APPENDIX A

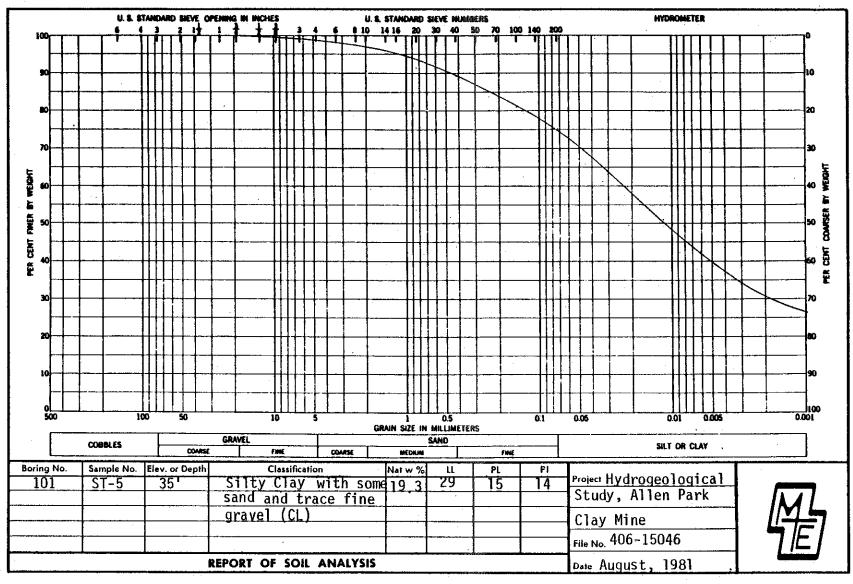
CURRENT LABORATORY TEST RESULTS

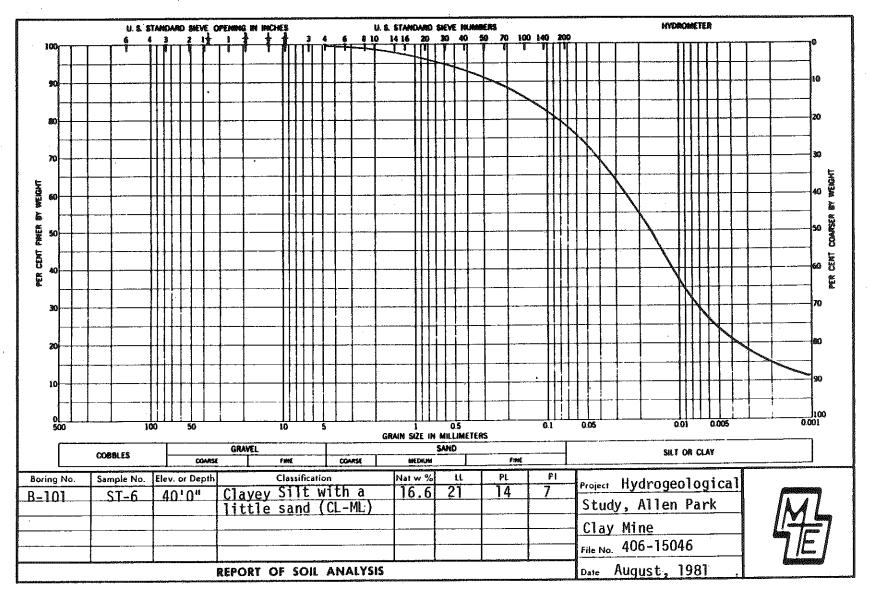
GRAIN SIZE PLOTS

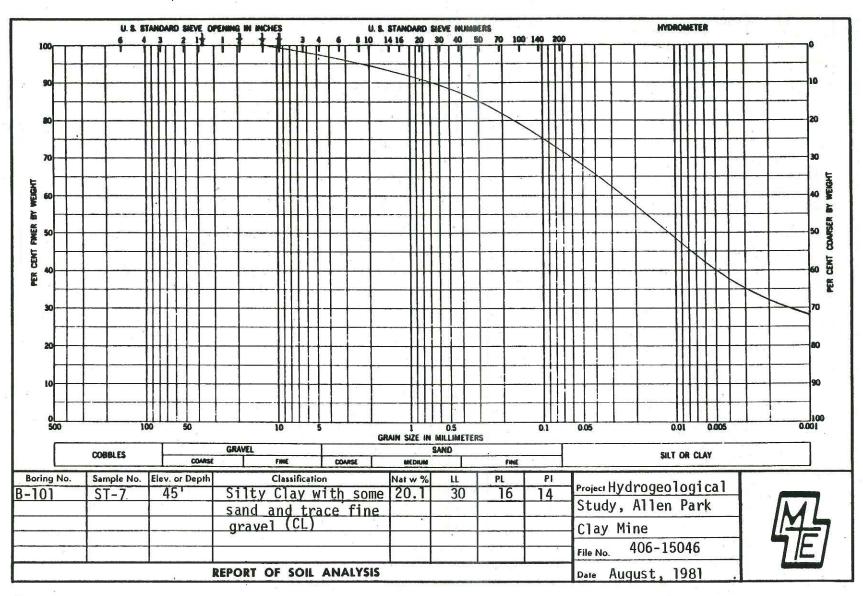


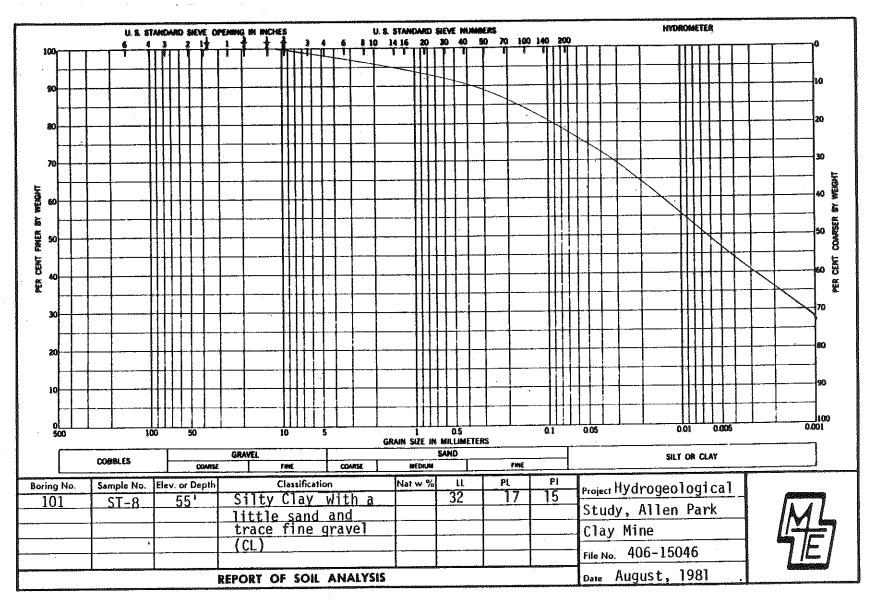


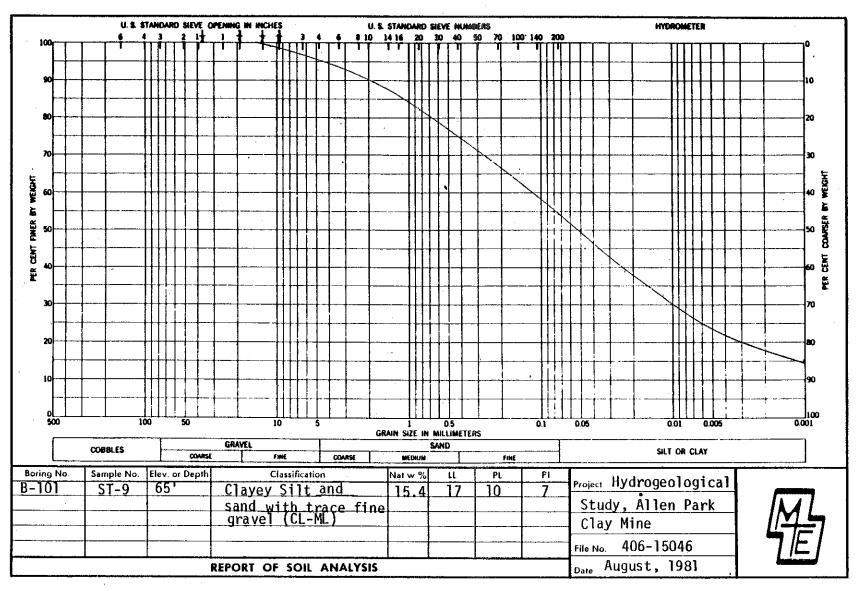


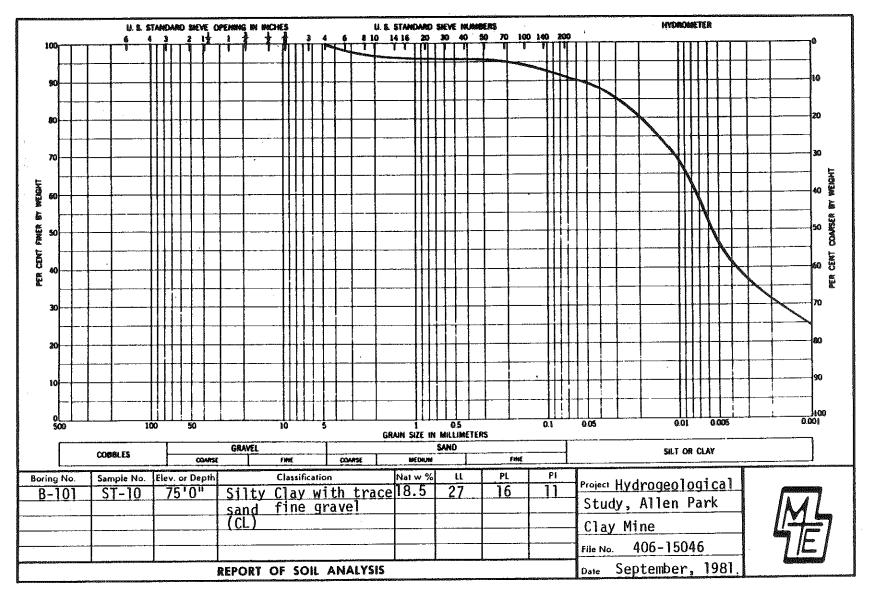


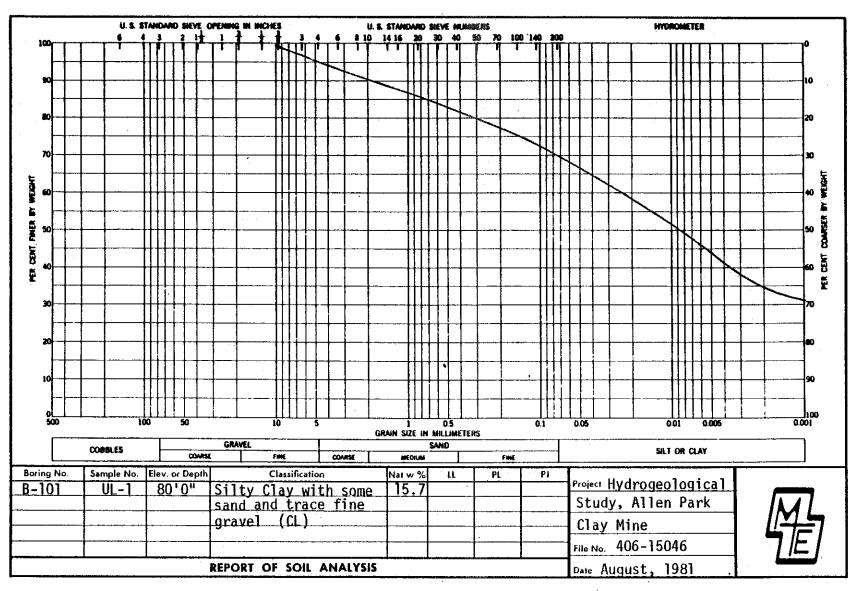


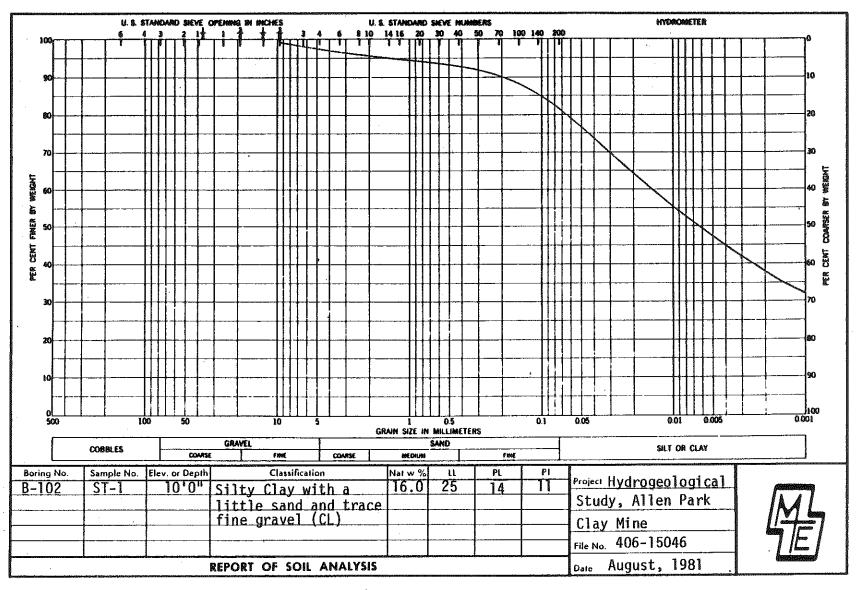


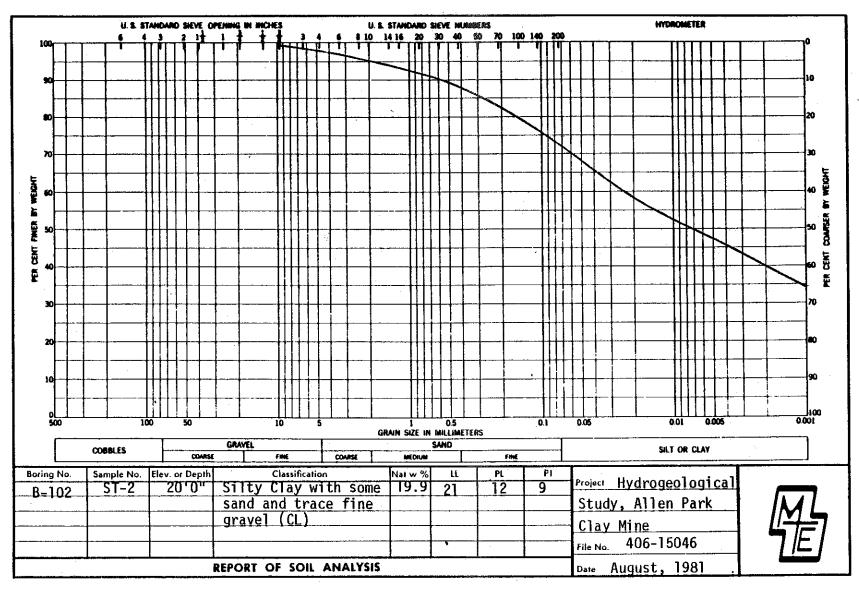


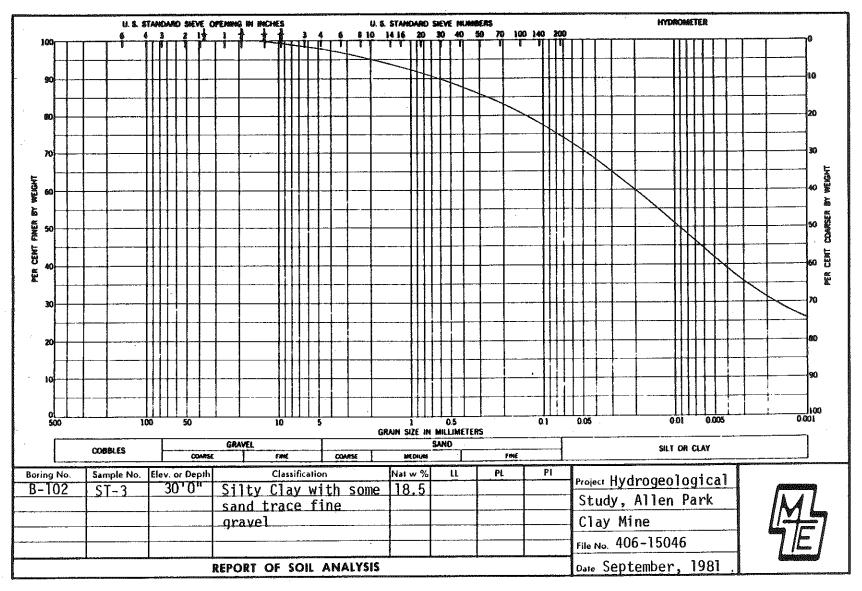


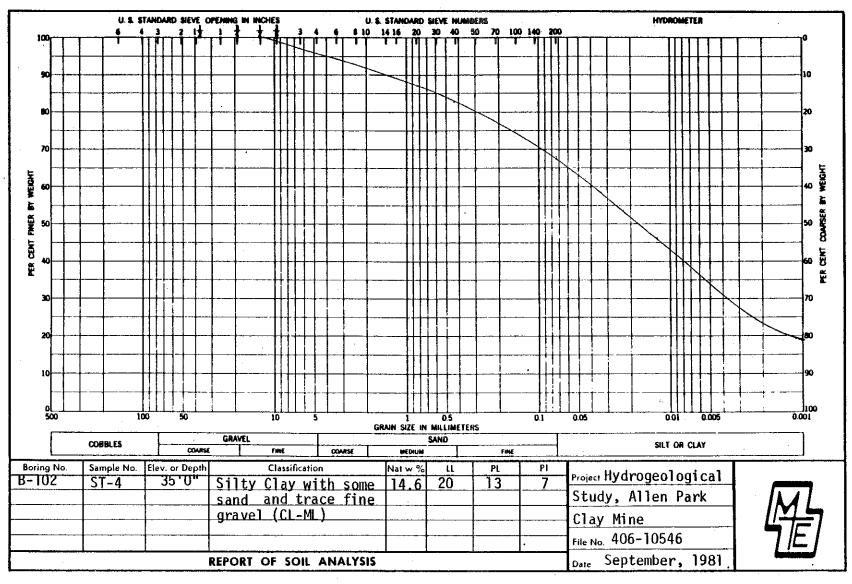


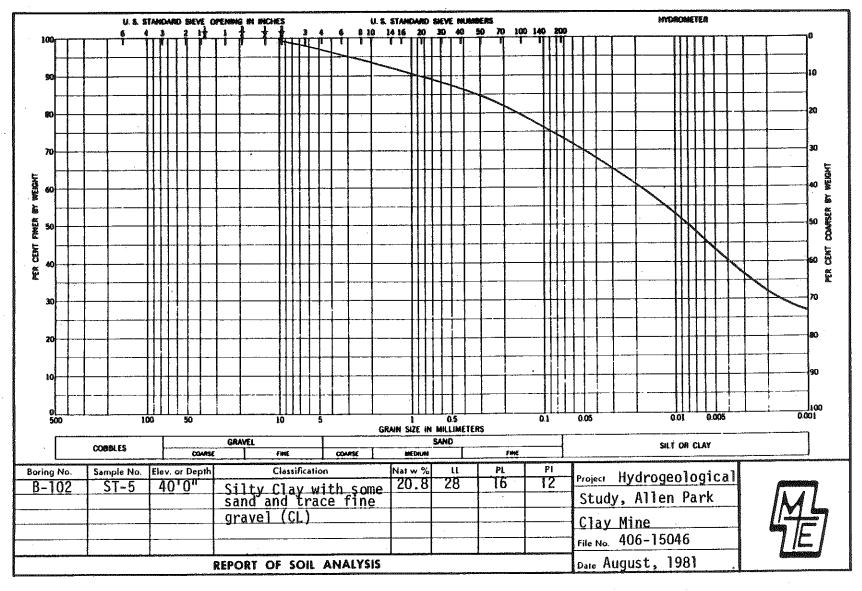


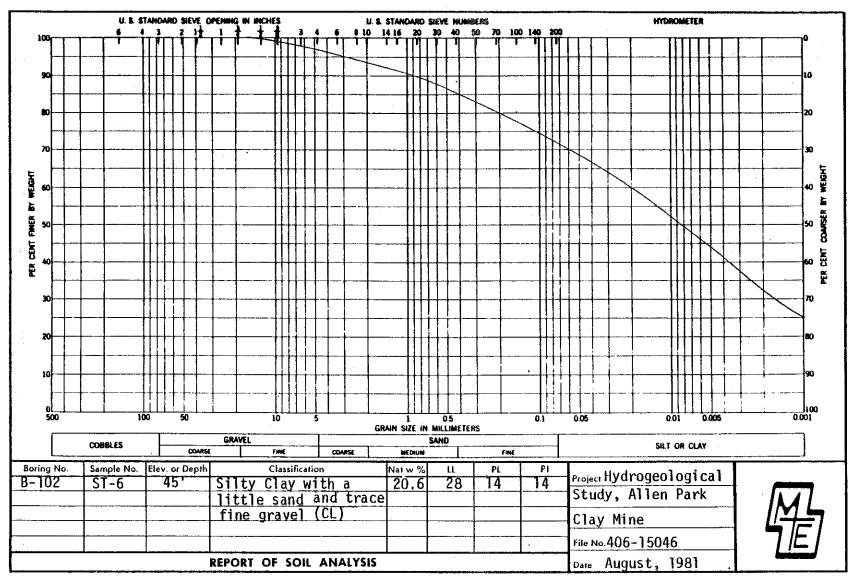


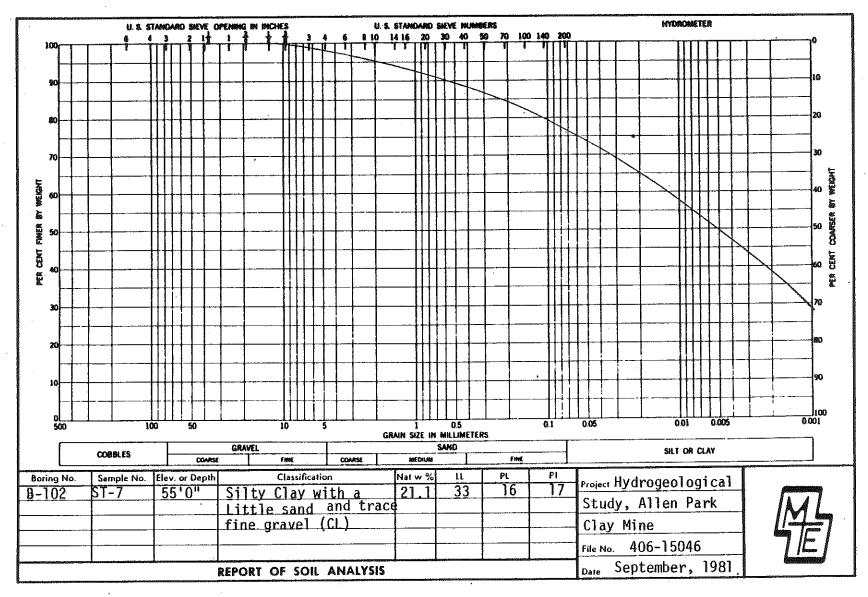


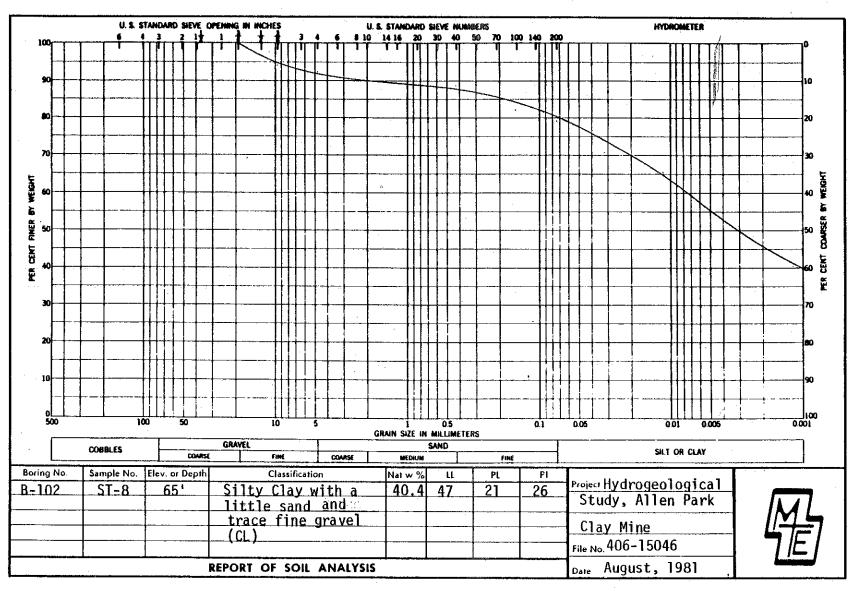


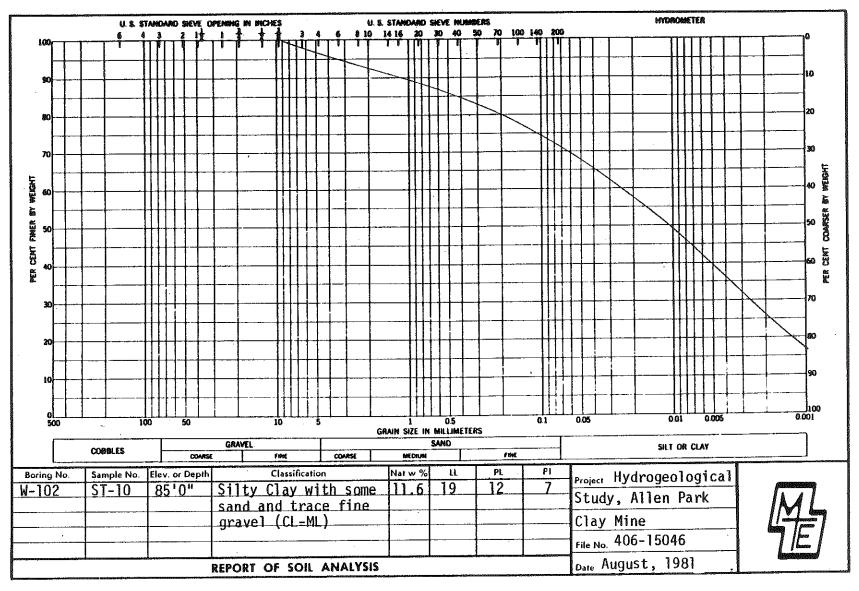


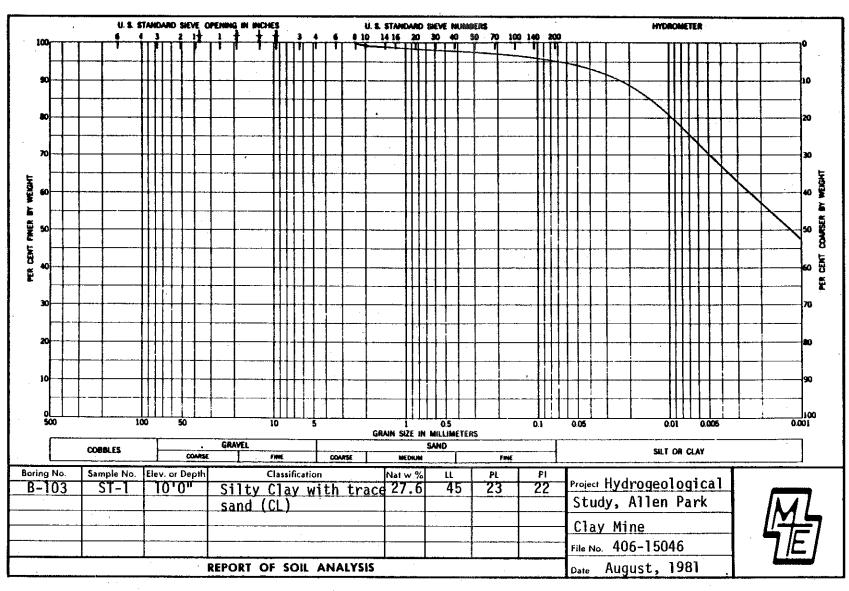


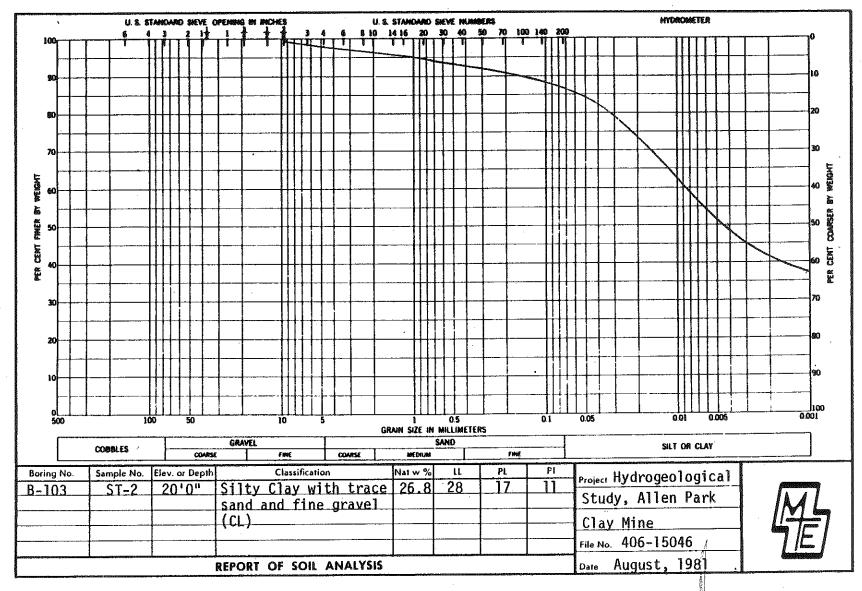


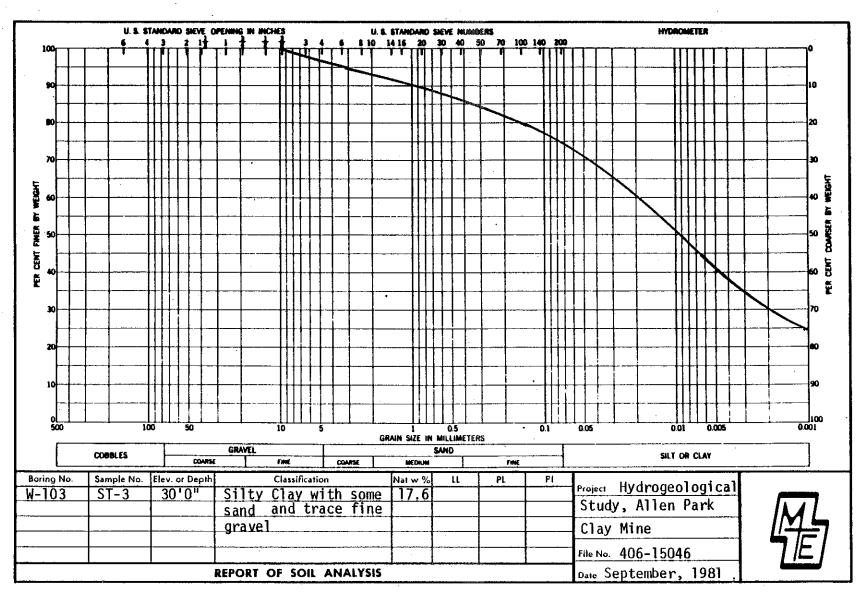


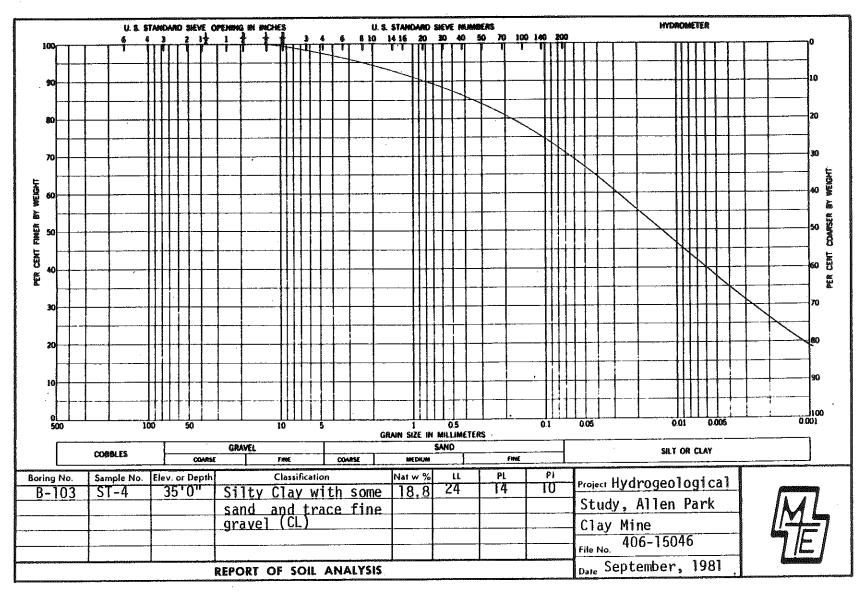


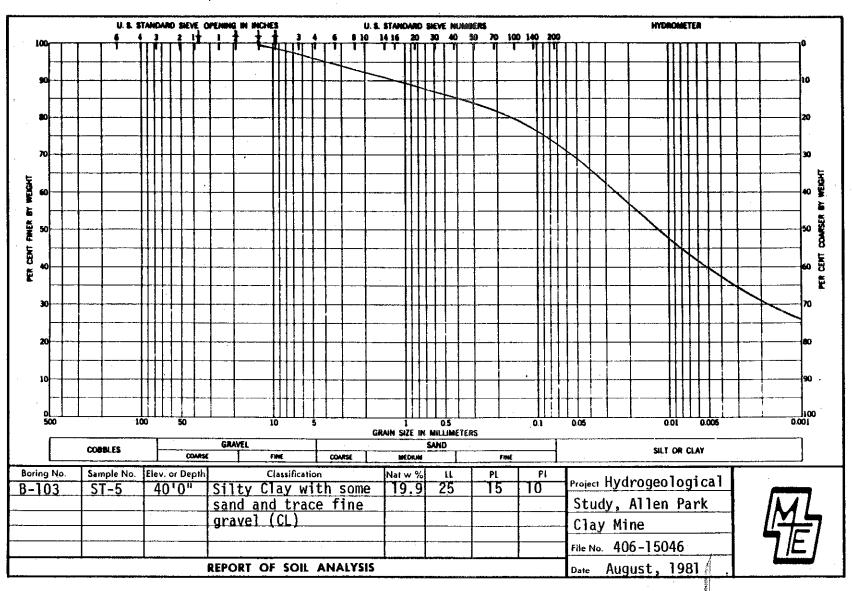


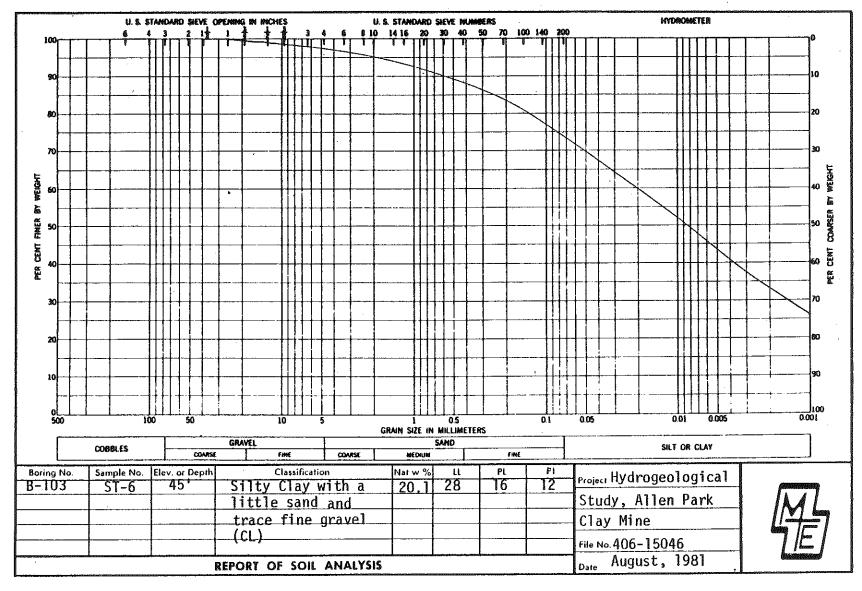


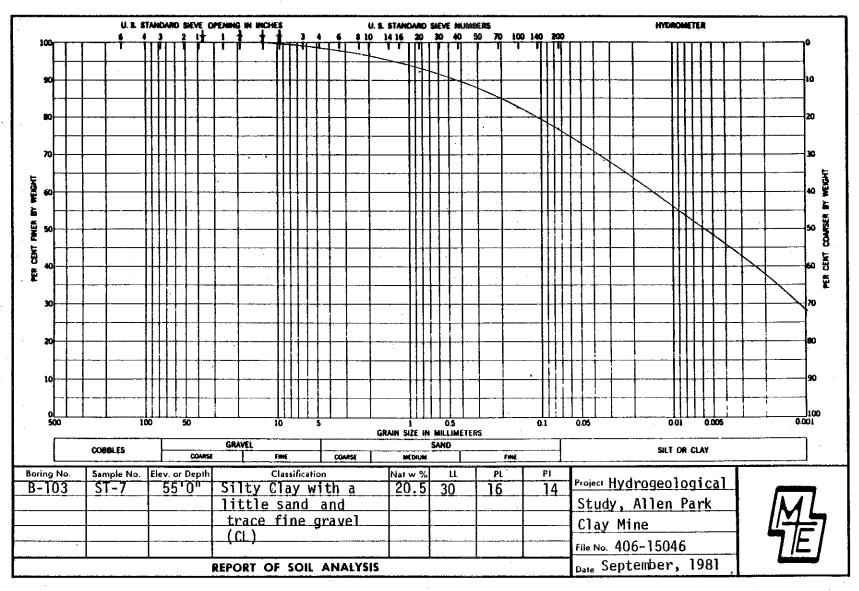




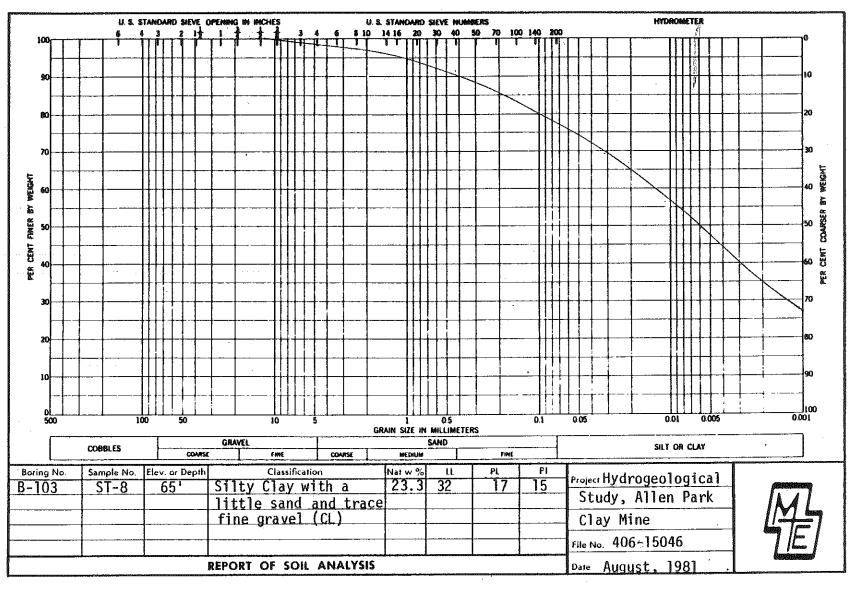


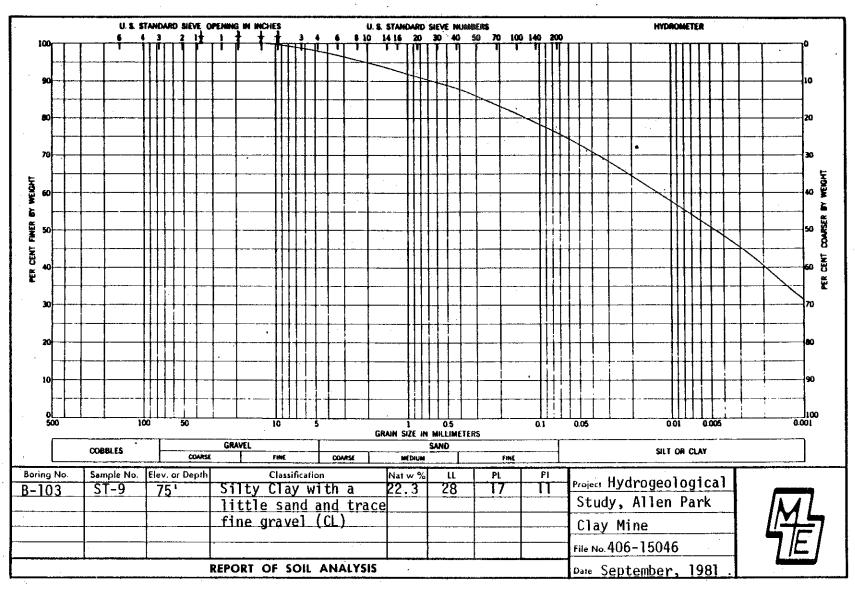


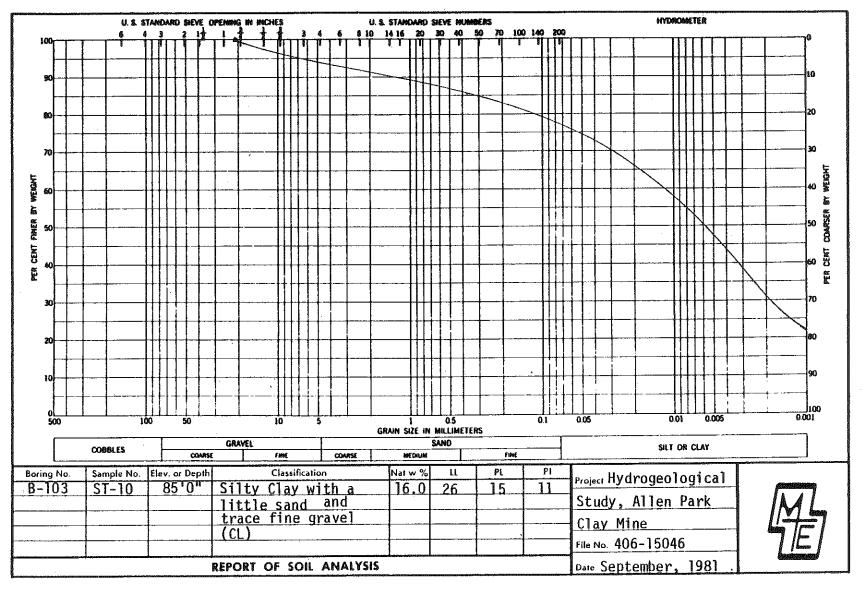


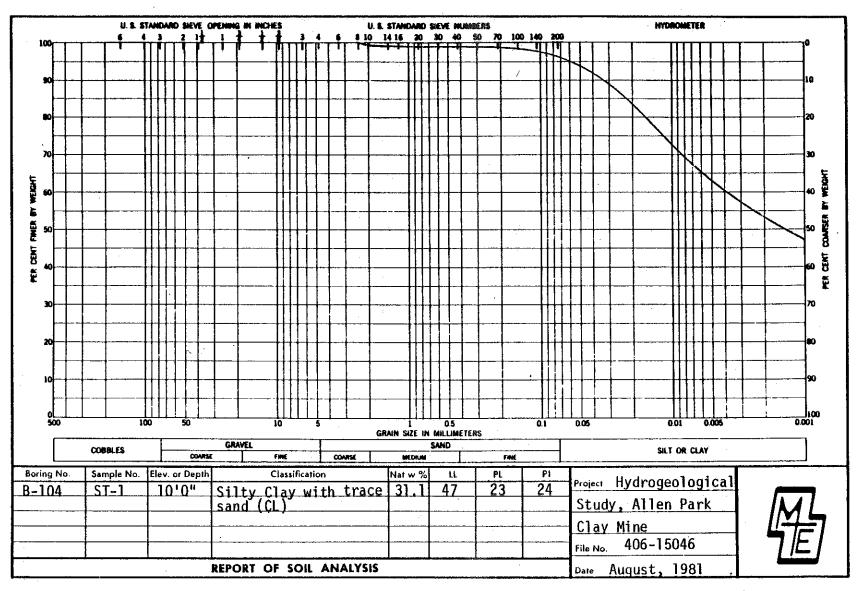


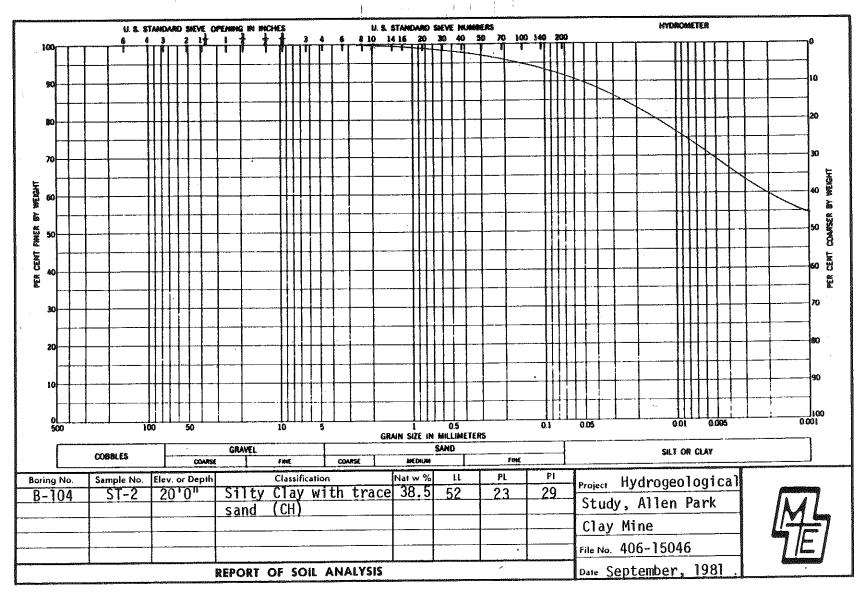
the state of the s

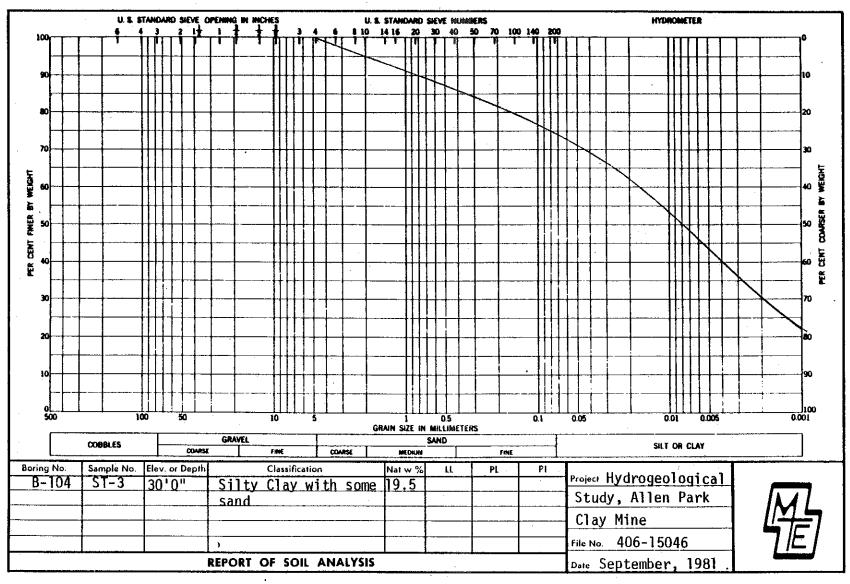


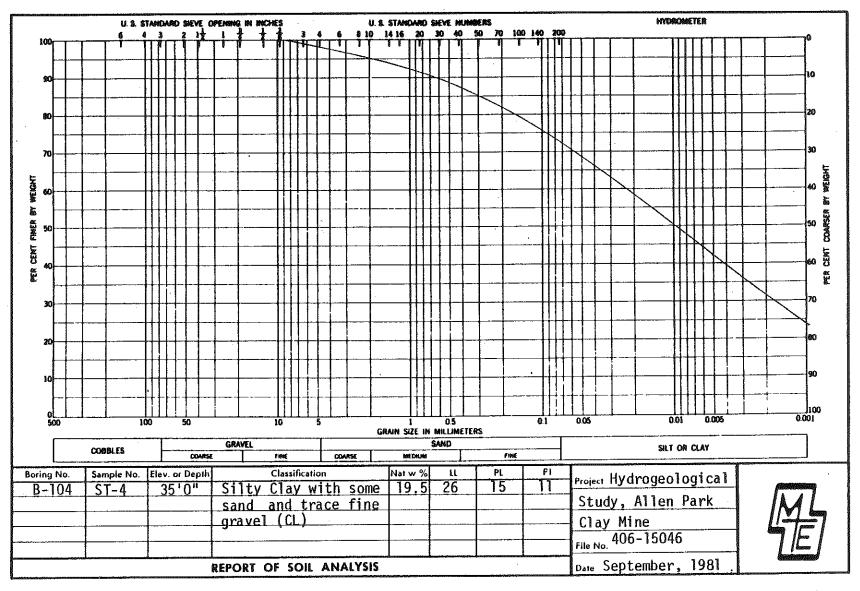


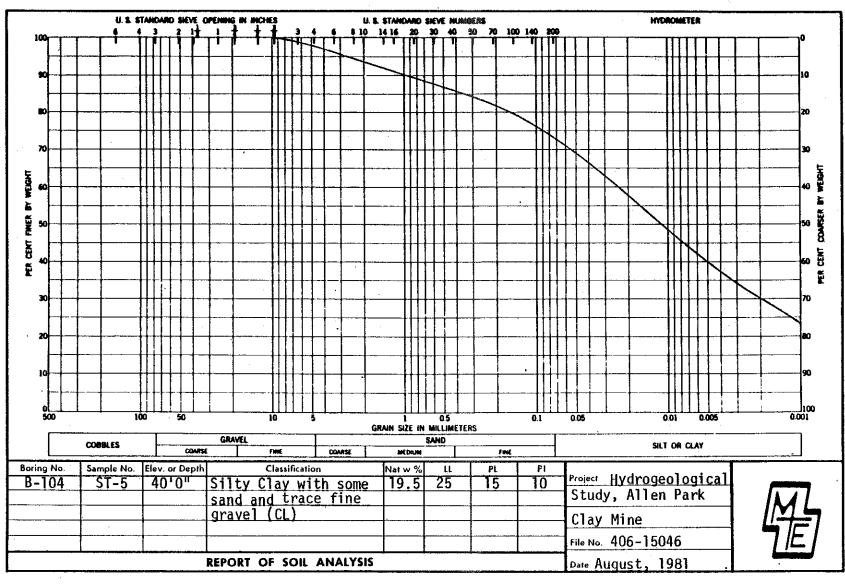


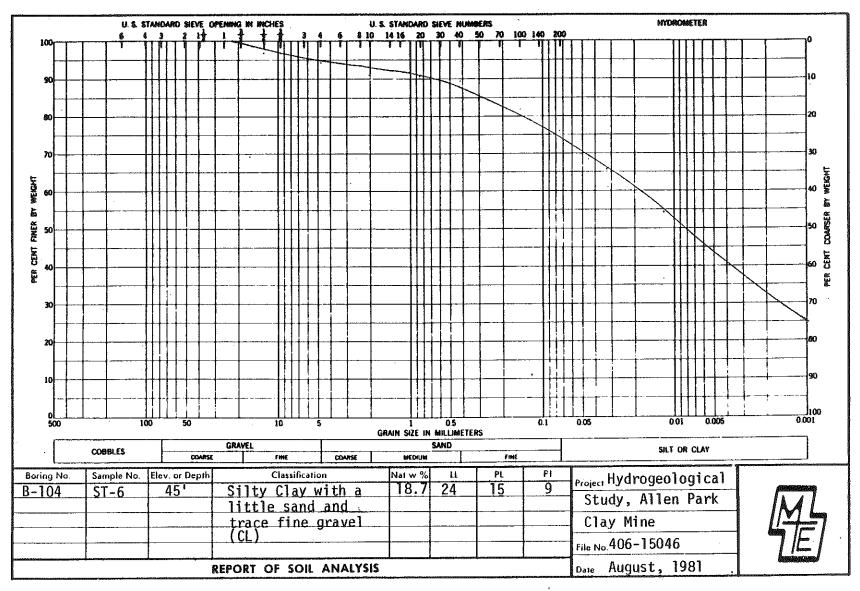


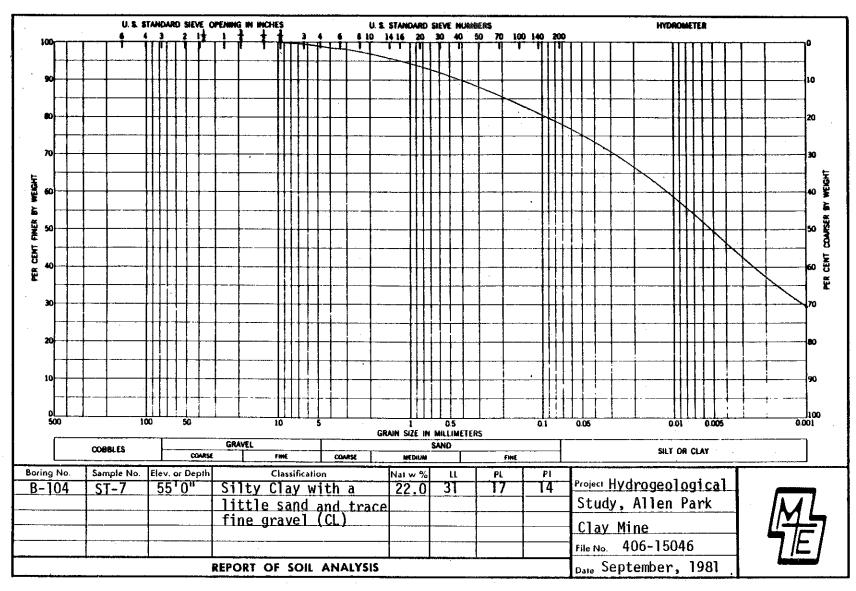


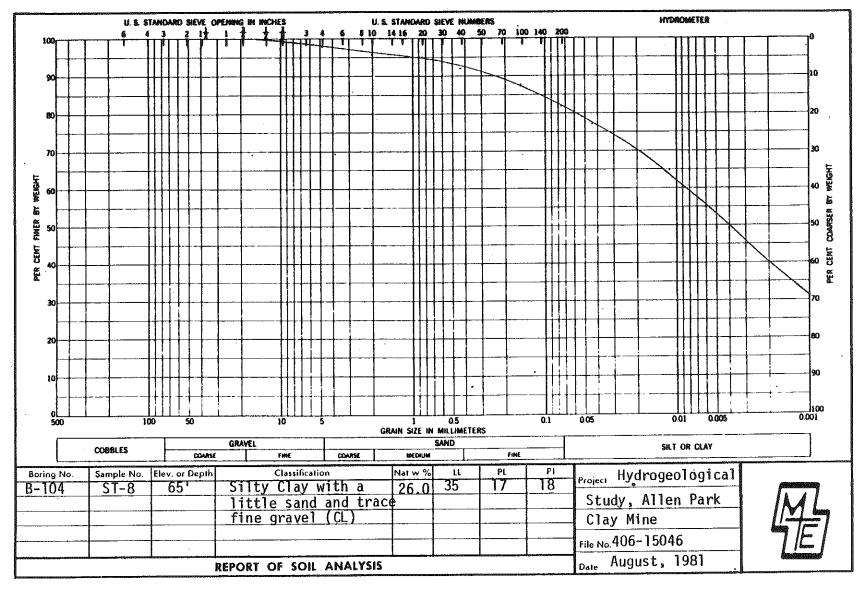


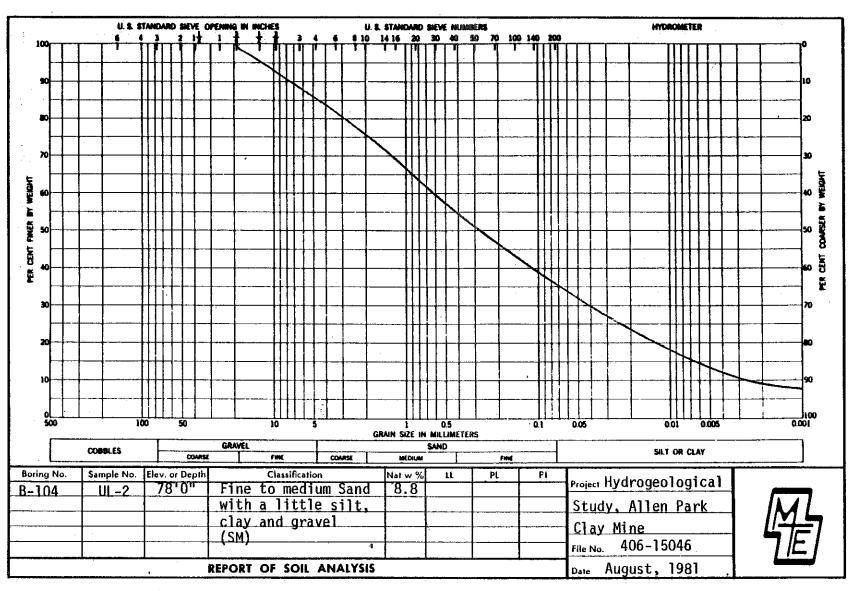


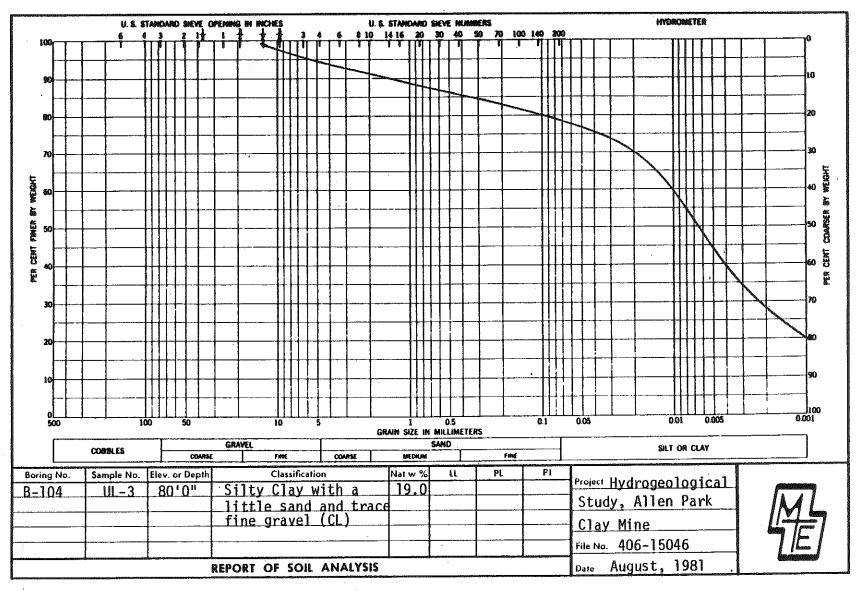


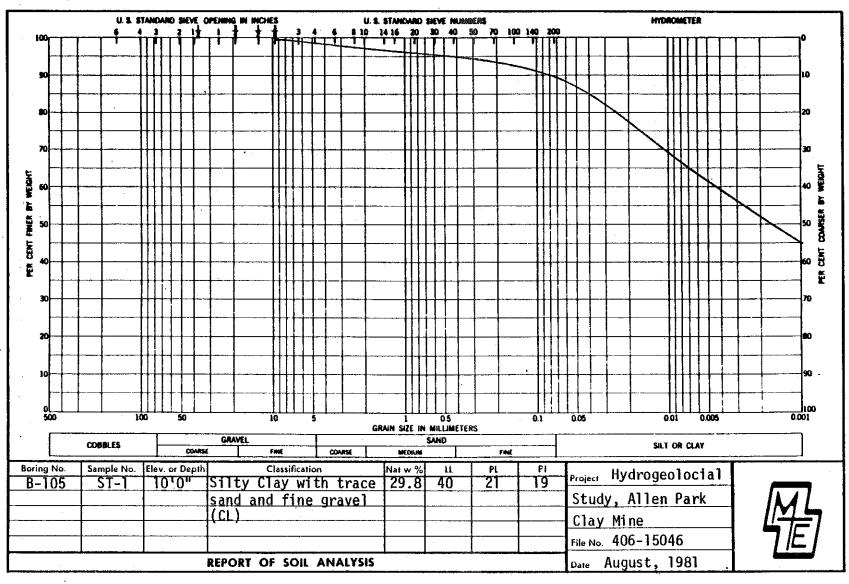


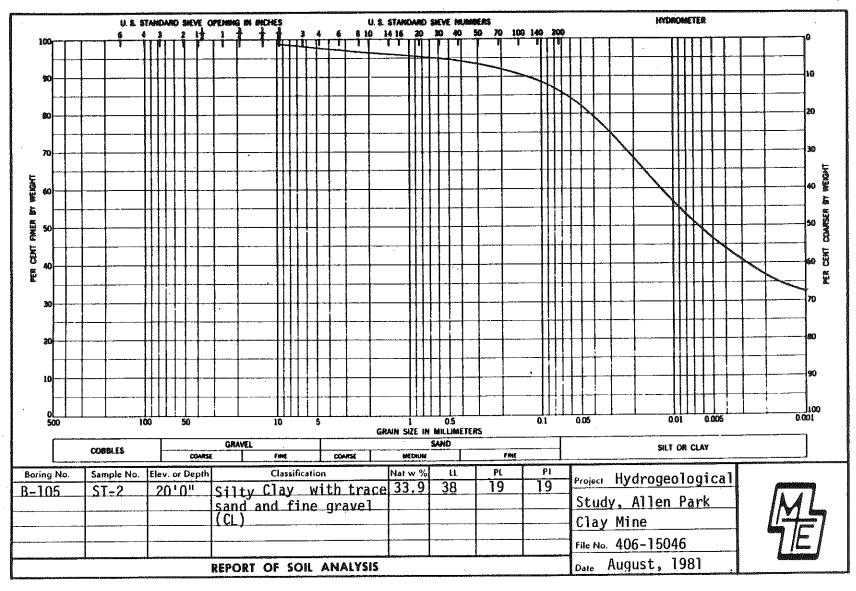


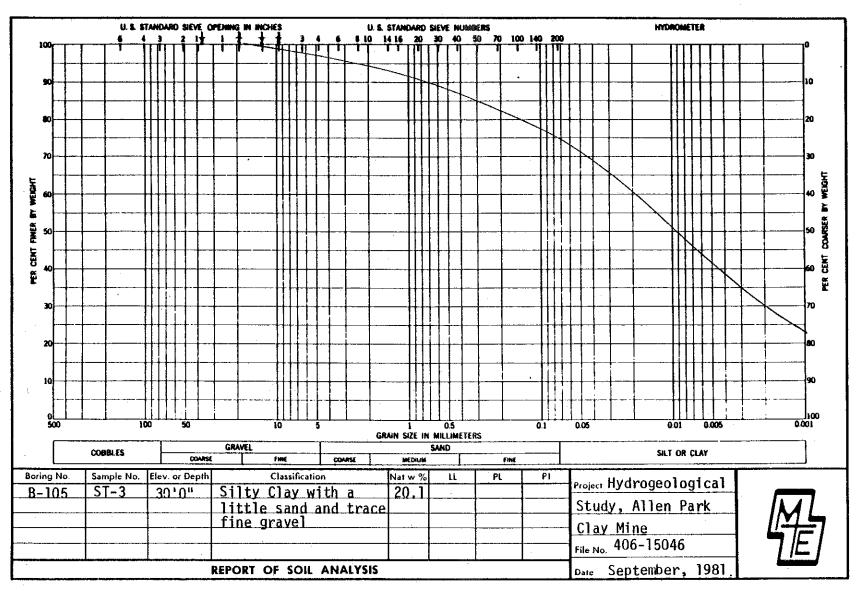


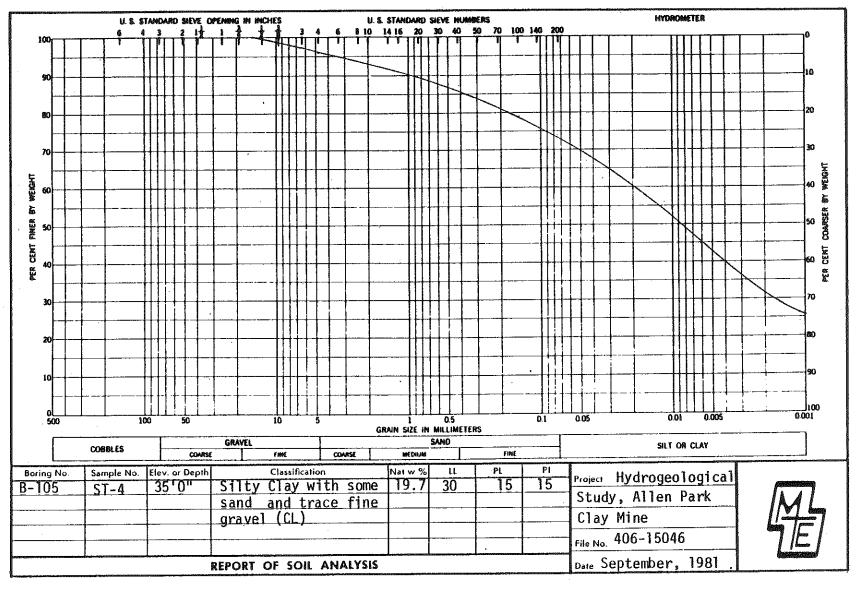




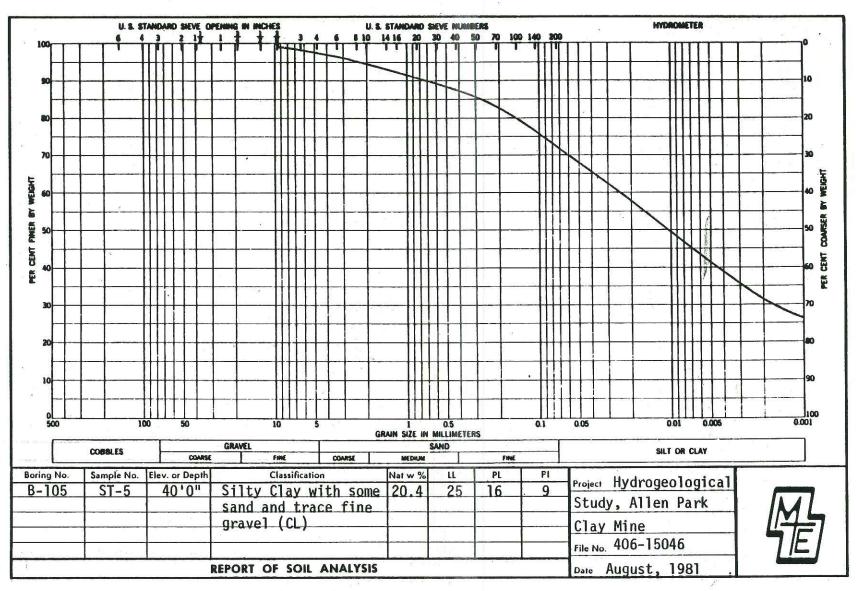


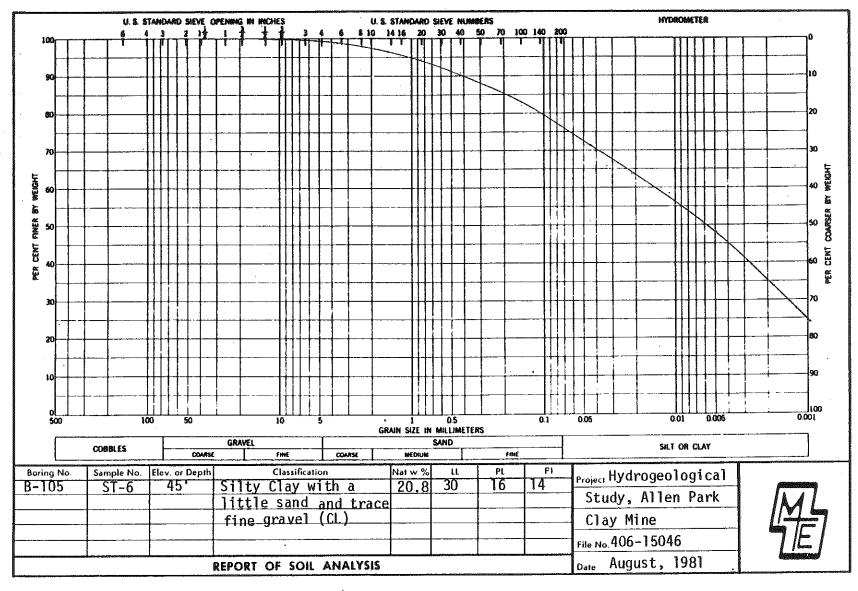


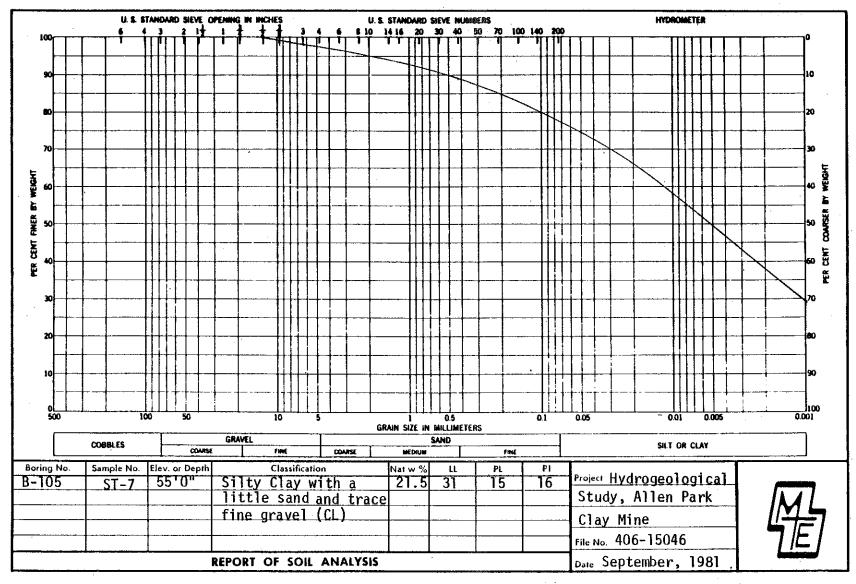




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MICHIGAN TESTING ENGINEERS. INC. -

APPENDIX B
SOIL BORING (WELL) LOGS CURRENT PROJECT

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# MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

406-15046

W-101 LOG OF SOIL BORING NO.

J08 NO.

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

DATE 8-6-81 +593.9

Allen Park, Michigan

9	SURFACE ELI	EV DATE 8-0-81			en Pari			
Sample Depth L	egend	SOIL DESCRIPTION	Penetration Blows For t	Moisture %	Natural Wt. P.C.F.	Ory Cen Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
0	'9"	Silty, clayey SAND with organics						
		(Topsoil) (CL-ML)						
2		Very silty CLAY, with trace sand,						
3 3	'0"	gray, moist (CL)					w.o	
4		Silty CLAY, with trace sand, gray,						
		moist (CL)	Accompany					
5	ů .			-				
6 6	'0"							
<del></del>		Fine to medium, SAND, brown, moist						
		(SP)						
8								
9								
10								
	.   7							
ST   11		No recovery	_				1	
12	]	1						
13 1	3'0"							
		Silty CLAY with trace sand and fin	e —					
14	1	gravel, gray, moist (CL)						
15		(1) 00 07 01		22.3	122.8	100.4		<del> </del>
ST 16		(LL=24, PI=8)						
17						-		
18	Ì		-					-
19								
ST 20	}	With thin fine sand partings		33 4	121.4	91.0		-
3	٦	(LL=36, PI=19)		30.1				
21								-
22								
23						<u>L</u>		
	ļ		1000					
24								
25			-					
TYPE OF SAMP	15 8	EMARKS:	Δ1	rtesian	around	Water	encount	<u>l</u> ered
D DISTURBE U.LUNDIST. LIV	D NER		a	t 75'6"				
S.T SHELBY T S.S SPLIT SPO	TUBE OON		1					
R.C ROCK COP ( ) - PENETRO	METER	Standard Penetration Test — Driving 2" 0D Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" intervals			Sheet	of 4		

406-15046

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_\_\_

W-101

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

JOB NO. \_

-		SURFACI	E ELEV. +593.9 DATE 8-6-81		A1	len Pa	rk, Mi	chigan	
Sampia & Typa	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural WL P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Sti %
	26	.							
	27		Silty CLAY with a little sand and trace fine gravel, gray, moist (CL	)					
	28								
	29								
ST	30		(!!-27 DT-11\		19.3				
4			(LL=27, PI=11)		13.3				
	31								
	32								
	33								
	34								
ST 5	35		(LL=29, PI=14)		19.3	129.8	108.8		
7	36							· · · · · · · · · · · · · · · · · · ·	
	37								
	38	-							
	39								
ST	40		(LL=21, PI=7)		16.6				
6	41								
	42								
	43		·						,
	44								<u> </u>
<u></u>	٠.		(1)		00.1	107.0	700 5		
ST 7	45		(LL=30, PI=14) ↓ Becoming silty clay (CL)		20.1	131.3	109.3		
	46		W Seconding Strict Clay (OL)						
	47								
	48								
	49								
	50								
ĬĀI	PE OF SAM	APLE BED	REMARKS:		<u> </u>		1		
U.L. S.T.	UNDIST. SHELBY	LINER / Tube			-		ř	•	_,
R.C.	SPLIT S ROCK C PENETF	ORE	Standard Penetration Test — Driving 2" 0D Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" intervals			Sheet	2 of 4		Ì



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

W-101 LOG OF SOIL BORING NO.

PROJECT Hydrogeological Study

406-15046 JOB NO..

LOCATION Allen Park Clay Mine

Sheet 3 of 4

DATE 8-6-81 +593.9 <u>Allen Park, Michigan</u> SURFACE ELEV. \_\_\_\_ Natural WL P.C.F. Ory Den Wt. P.C.F. Unc. Comp. Strength PSF. Penetration Blows For 6" SOIL DESCRIPTION Depth Legend 51 Silty CLAY with a little fine sand and trace fine gravel, gray, moist 53 54 55 21 9 (LL=32, PI=15)56 57 58 59 60 61 62 63 64 Becomming clayey silt with fine sand 15.4 ∳ (CL-ML) (LL=17, PI=7) 66 67 68 69 70 Becomming very silty clay (CL) 72 74 (Shelby Tube refusal at 75 Ft.) ST 10 (LL=27, PI=11) 18.5 REMARKS: TYPE OF SAMPLE D. - DISTURBED U.L.-UNDIST. LINER S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE

Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

( ) - PENETROMETER

15046 W-101 PROJECT Hydrogeologica SURFACE ELEV. +593.9

Sample   SURFACE ELEV. +593.9	PROJECT Hydrogo	007
000	- IOCATIO AZO	≝010gici _
DATE 8-6-01	LOCATION Allen P	ark Clay
O CLAV CLAV CLAV	Allen P	ark, Mic i
77 77'0" Medium Sand and occa-	Penetration Moisture Natural	AIR, Mic i
	Slows For 6" Moisture Natural Wt. P.C.F	Ory Den L W. F. Sti
77 77'0" Medium SAND with trace fine gravel, gray, saturated (SP)  78 78'0" Silty, sandy CLAY		<del></del>
		<del></del>
Silty CLAY with		
fine gravel with some sand and	<del></del>	
Silty CLAY with some sand and trace (CL)  81  Silty CLAY with some sand and trace (CL)  Silty CLAY with some sand and trace (CL)		
36177	8	
82 Boring terminated at 80'6"	9 15.7	
83 80'6"		
$\frac{1}{84}$		
85		-
		+
Well Data:		
Well screen tip set at depth of  88  88  Well screen tip set at depth of  88	+	-
78'3" below existing ground level.		+
88   String ground level		<del>-</del>
#601 47 of top of pyc		<del></del>
standpine.		
190		
91		
<del>    92</del>		
93		
95		
	+	
$-\frac{96}{}$		
97		
	+	
98		
99		
TYPE OF SAMPLE D DISTURGE REMARKS		<del></del>
UI JUNIO CHED		$\frac{1}{1}$
SS COULD TUBE		+
DESTRUCTION CORE		<del> </del>
Standard Penetration Test — Driving 2" OD Sampler 1' With Sheet		
Falling 30", Count Made At S" Distance 1' With		7
Sheet	4 of 4	$J_{1}$
		7
	The state of the s	1



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

JOB NO. \_

LOG OF SOIL BORING NO.

W-102

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

406-15046

+501 3

0\_7\_01

Allen Dawk Michigan

		SURFACI	ELEV. +591.3 DATE 8-7-81		·····	Al	1en Pa	rk, Mi	<u>chigan</u>	
Sample & Type	Depth	Legend	SOIL DESCRIPTION	Pen Blow	etration 's For 6"	Moisture %	Natural Wt. P.C.F.	Ory Oan Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
	1	0'9"	Silty SAND with trace clay and	7				<u>, , , , , , , , , , , , , , , , , , , </u>		
	2		vrqanics (Topsoil) (SM-ML) Fine to medium, brown, moist SAND	_/						
	3	3'0"	(SP) ↓Wet					3/220-13-14-14-14-14-14-14-14-14-14-14-14-14-14-		
			Ciltu CIAV with Javana of Sing and			***************************************				
	4	!	Silty CLAY with layers of fine sand gray, moist, stiff (CL)	J,						
UL.	5					15.6	Militar Constitution Co.			
	6	6'0"								
	7		Silty CLAY with a little fine sand							
	8		and trace fine gravel, gray, moist (CL)							
	9									
ST	10		(LL=25, PI=11)		.,,	16.0				
			(LL-23, FI-11)							
	11									
	12									
	13									
	14:									
	15									
	16									
	17	•								
	18									
	19									
		-	(L) 07 DT 0			30.0				
ST 2	20		(LL=21, PI=9)			19.9			-	
	21									
	22									
	23									
	24	_								
	25	<i>.</i>								
TVE	E OF SA	MPI E	REMARKS:	·······	Gre	Jund w:	ter er	Counte	red at 2	Feet
D. U.L.	- DISTU -UNDIST.	rbed Liner	-		Art	tesian	ground		encount	
S.S. R.C.	SHELE SPLIT ROCK	SPOON CORE	Standard Penetration Test — Driving 2" OD Sampler 1' With		dt	86 fee		£		
( )	- PENET	ROMETER	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals		<u></u>	<u>Sne</u>	et l o	T 4		

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_

W-102

406-15046

JOB NO. \_

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

Allen Park Michigan

		SURFACE	ELEV. +591.3 DATE 8-7-81		.A11	en Par	k, Mic	nigan	
Sample & Type	Bepth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Oen Wt. P.C.F.	Unc. Comp. Strength PSF.	Stı %
	26		0:14 01 AV						
	27		Silty CLAY with a little fine sand and trace fine gravel, gray, moist						
	28		(CL)						
	29								
ST	30				18.5				<u> </u>
3	31		(LL=24, PI=9)						
	32								
	33		T (CL-ML)						
	34								<u> </u>
ST	35		(LL=20, PI=7)		14.6				
4	36	•	:	,					
	37								<u> </u>
	38								
	39		√ (CL)						
ST	40		(LL=28, PI=12)		20.8				
5	41	·		-					
	42		•						<u> </u>
	43								
	44								<u></u>
ST	45		(LL=28, PI=14)		20.6	128.2	106.3		
6	46	!							
	47								
	48								
	49								+
	50	. •							<del>                                     </del>
TYP	E OF SAN	APLE	REMARKS:		1	<u></u>		i	<u> </u>
U.L. S.T.	- DISTUR - UNDIST - SHELBY - SPLIT S	Liner / Tube				•			J
R.C.	- ROCK C - PENETR	ORE	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" intervals		Sheet	2 of	4		



# MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

406-15046

JOB NO. .

+591.3 SUBFACE ELEV

DATE 8-7-81

LOG OF SOIL BORING NO. .

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

Allen Park, Michigan

W - 102

			SURFACE	ELEV. +591.3 DATE 8-7-81		AII	en Par	k, Mic	higan	
Sa &	mple Type	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Oen Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
		51								
				Silty CLAY with a little fine sand						
-		52		and trace fine gravel, gray, moist (CL)						
		53								
_		54								
-	ST	55		(LL=33, PI=17)		21.1				
-	7	56								
_										
-		57							-	
		58						and the state of t		
-		59		·						
	_	60		•						
		61								
		62								
		63	-	With a trace fine sand						1.
		64	-	•						
-	ST	65		(LL=47, PI=26)		40.4	108.3	77.1		
-	8	66								
_		67								
				• ,						
		68								
F		69								
_		70	1	. •						
		71								
F		72	]	·	-					
-			†							
		73	1	T Becomming very stiff						
-	ST	74	1	(Shelby tube refusal at 75 Ft.,						
F	9	75	]	No sample recovery)		***				
	D. U.L. S.T. S.S R.C	E OF SA - DISTUR - UNDIST. - SHELB - SPLIT - ROCK	RBED LINER Y TUBE SPOON CORE	REMARKS:  Standard Penetration Test — Driving 2" 0D Sampler 1' With		Sh	leet 3	of 4	and we consider the second	
	( )	- PENET	ROMETER	Standard Penetration Test — Driving 2" 00 Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals		J1		<b>∵</b>		



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

W-102 LOG OF SOIL BORING NO. \_\_\_

Hydrogeological Study

Allen Park Clay Mine LOCATION .

406-15046 JOB NO. \_

		SURFACE	ELEV. +591.3 DATE 8-7-81	·	A1	len Pa	rk, Mi	chigan	
Sample & Type	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	itr. %
	76		Silty CLAY with a little fine sand						
		n =:	<pre>and trace fine gravel, gray, moist, very stiff (CL)</pre>						
	77		very 30111 (02)	-					
	78						=		
	79	, a							
UL	80	•	(LL=35, PI=15)	<u>5</u>	24.2				
2			(LL-35, F1-15)	-10				_	
	81							A .	
	82	9 3							
	83						·		
	84		▼ (CL-ML)						
			(LL=19, PI=7)		11.6				
ST 10	85		(Shelby tube refusal at 85'6")		11.0				
	86	86'0"							
	87		Medium SAND with trace fine gravel						
	88		gray, saturated (SP)						
	89								
	90	2 **							
	91	91'0"				1			
-	92		Silty SAND and clay, hard, slightly	,					
		5	moist (Hardpan)						
	93	93'6"	* * * * * * * * * * * * * * * * * * *						
	94		Boring terminated at 93'6"						
	95								
	96	, a	Well Data:						
	97		Well screen set at 93 feet below existing grade.						
		1 -	* ***						
	98	1	Elevation of top of PVC standpipe: +600.81						
	99	1	* , to					1	I
	100	1							
TW	PE OF SA	MP1 F	REMARKS:						Ь_
D. U.L S.T S.S	- DISTUR UNDIST. SHELB S SPLIT C ROCK	RBED LINER Y TUBE SPOON	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals		SI	heet 4	of 4		



W-103 LOG OF SOIL BORING NO. .

PROJECT Hydrogeological Study CONSULTING ENGINEERS IN SOILS & FOUNDATIONS LOCATION Allen Park Clay Mine 406-15046 JOB NO. ... +593.9 DATE 8-10-81 Allen Park, Michigan SURFACE ELEV. Penetration Blows For 6" Dry Den Wt. P.C.F. Unc. Comp. Strength PSF. Maisture Natural WL P.C.F. Sample & Type SOIL DESCRIPTION Legend Depth 0'7" Silty SAND with trace clay and organics (Topsoil) (SM-ML) 2 Fine to medium SAND, brown, moist (SP) 4 Wet 5 6 8'0" 8 9 Silty CLAY with a little sand, gray, moist (CL) 10 27.1 (LL=45, PI=22)12 13 14 15 16 With trace fine gravel 18 19 20 (LL=28, PI=11) 26.8 ST 21 2 22 23

TYPE OF SAMPLE D. - DISTURBED

24

25

U.L.-UNDIST. LINER S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE

( ) - PENETROMETER

REMARKS:

Standard Penetration Test — Driving 2" 0D Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" intervals

Ground water encountered at 4'0". Artesian ground water encountered at 85 feet.

Sheet 1 of 4

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TE	

406-15046

.og	OF	SOIL	BORING	NO.	

M-	Ц	13	

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

JOB NO. \_

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

		SURFACE	ELEV. +593.9 DATE 8-10-81		A11	en Par	k, Mic	higan	
Sample & Type	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
	26	-							
	27		Silty CLAY with some sand and trac	e					<u> </u>
	28		fine gravel, gray, moist (CL)						
	29	. [						•	
	30	randos promo							
ST					17.6				
3	31		(LL=25, PI=10)						
	32								
	33								
	34		• .						
	35						-		
ST	36		(LL=24, PI=10)		18.8				
4	37								
	38								
	39								
	40								
CŦ		* 3. *			19.9				
ST 5	41		(LL=25, PI=10)		13.3				
	42			-					
	43			-					
	44								
	45								
ST	46		(LL=28, PI=12)		20.1	130.8	108.9		
6	47								
	48			,					
	49						<u> </u>		<del> </del>
	50				-	<del>                                     </del>			
		nt 2	REMARKS:		<u> </u>				
D. U.L	<b>PE OF SAM</b> - Disture - Undist. L	BED JINER	nanzura						
S.T S.S R.C	SHELBY SPLIT S ROCK CO PENETR	TUBE POON DRE	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" intervals			2 of 4	1		



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_\_\_

Hydrogeological Study

Allen Park Clay Mine LOCATION \_

406-15046

J0B NO. \_\_

		SURFACE	.500.0			len Pa	rk, Mi	chigan	
Sample & Type	Depth	Lagend	SOIL DESCRIPTION	Penetration Blows For 6	Moisture %	Natural Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Como. Strength PSF.	Str. %
	51								
	52		Silty CLAY with a little sand and trace fine gravel, gray, moist (CL	)					
	53							_	
	54							-	
	55_			*******************************					
CT			/U = 20 DT = 1.6 \		20 5				
ST 7	56		(LL=30, PI=14)		20.5				
	57	. ,							
	58					!			
	59								
-	60								
	61								
	62								
	63								
	64			<u>-</u>					
	65		and the second s						1
ST	66	* 30. **	(LL=32, PI=15)		23.3	127 2	103.2		
8			\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\		23.3	127.2	103.2		
	67	<u> </u>							
	68								
	69								
	70								
	71_		•						
	72								
	73	4							
	74	1	·						
,	75								
TV	PE OF SA	MPLF	REMARKS:						
D. U.I S.1 S.2	- DISTU LUNDIST T SHELE S SPLIT C ROCK	rbeo . Liner 3y tube . Spoon	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" intervals		annan concern	Sheet :	3 of 4		

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

406-15046

JOB NO. ..

PROJECT Hydrogeological Study

LOG OF SOIL BORING NO.

W = 103

LOCATION Allen Park Clay Mine

+593.9 DATE 8-10-81 Allen Park, Michigan SURFACE ELEV. Moisture % Unc. Comp. Strength PSF. Penetration Blows For 6" Naturai Wi, P.C.F. Dry Den Wt. P.C.F. Sampie & Type Depth Legend SOIL DESCRIPTION 22.3 76 Silty CLAY with a little sand and 9 trace fine gravel, gray, moist(CL) (LL=28, PI=11) 78 80 81 82 83 84 10 85 | 85 '0" (LL=26, PI=11) 16.0 Medium SAND with a trace fine gravel, 86 gray, saturated (SP) 87 88 89 90 91'0" 91 Silty SAND and clay, hard, slightly moist (Hardpan) 92 93 93'0" Boring terminated at 93'0" 94 Well Data: 95 Well screen set at 92'6" 96 below existing grade 97 Elevation of top of PVC standpipe: +605.06 98 99 100 REMARKS: TYPE OF SAMPLE

- DISTURBED

U.L.-UNDIST. LINER

S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE ( ) - PENETROMETER

Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

Sheet 4 of 4

# MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_

W-104

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

406-15046 JOB NO. \_

	SURFACE E	LEV. +594.1 DATE 8-12-	81	<u>A1</u>	len Pai	rk, Mic	higan	
Sample Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural WL P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
	7'4"	Silty CLAY with trace sand, dark brown, moist with organics (Tops	oil)					
2		Silty CLAY with a little sand, mottled brown and gray, moist (C						
UL 5				26.3				
6								
9								
ST 10 1 11	70'0"	(LL=47, PI=24)		31.1				
12		Silty CLAY with a little sand, g moist (CL)	ray,					
14								
16								
18		T Becomming more plastic (CH)						
ST 20 2 21		(LL=52, PI=29)		38.5				
22								
24	-							
TYPE OF SA  D DISTUI U.LUNDIST S.T SHELE S.S SPLIT R.C ROCK ( ) - PENET	RBED LINER SY TUBE SPOON CORE	REMARKS:  Standard Penetration Test — Oriving 2" 0D Sampler 1' W 140# Hammer Falling 30"; Count Made At 6" Intervals	at	73 fee	ground t t l of		encounte	ered

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

406-15046

PROJECT Hydrogeological Study

W-104

LOG OF SOIL BORING NO. \_

LOCATION Allen Park Clay Mine

\_\_\_ DATE\_8-12-81 +594.1 SUBFACE FLEV

Allen Park, Michigan

		SURFAU	E ELEV. +394.1 DATE 8-12-81		Allen Park, Michigan				
Sample: & Type	Depth	Legend	SOIL DESCRIPTION P	enetration owa For 6"	Moisture %	Natural Wt. P.C.F.	Dry Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str.
	26		Ţ (cr)			,			-
	20								
	27	-							
	28		Silty CLAY with a little sand and trace fine gravel, gray, moist (CL)						
	29	•	brace rine gravers, gray, morse (02)	,					-
					10 5				
ST 3	30		(LL=25, PI=10)		19.5			**************************************	
	31	•							
_	32								
			•						
	33								
	34							· ·	
ST	35		(LL=26, PI=11)		19.5				
4	36								
	37								
	38			-					
	39								
ST	40		(LL=25, PI=10)		19.5			-	
5		•	(22 25, (1 75)						
	41								
	42								
	43							-	
				,				,	
	44								
ST 6	45		(LL=24, PI=9)		18.7	130.0	109.5		
0	46								
	47								
		•							
	48								
	49								
	50								
	-		REMARKS:						
0.	PE OF SAN - DISTUR - UNDIST.	BED	ILIMATICA.						-
S.T. S.S.	- SHELBY	/ TUBE SPOON		She	eet 2 c	of 4			
R.C	- ROCK C	ORE ROMETER	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals		-				



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

W - 104LOG OF SOIL BORING NO.

Hydrogeological Study PROJECT \_

406-15046 JOB NO. \_

LOCATION Allen Park Clay Mine

Sheet 3 of 4

+594.1 Allen Park, Michigan DATE 8-12-81 SURFACE ELEV. Natural Wt. P.C.F. Ory Den Wil P.C.F. Unc. Comp. Strength PSF. Penetration Blows For 6" Moisture % Sample & Type SOIL DESCRIPTION Depth Legend 51 Silty CLAY with a little sand and trace fine gravel, gray, moist (CL) 52 53 54 22.0 (LL=31, PI=14)ST 55 56 57 58 59 60 61 62 63 64 26.0 124.8 99.0 (LL=35, PI=18) 65 66 67 68 69 70 72 73'0" 73 Silty, sandy CLAY, very stiff to hard (HARDPAN) 74 75 TYPE OF SAMPLE

0. - DISTURBED

U.L.-UNDIST. LINER

S.T. - SHELBY TUBE

S.S. - SPLIT SPOON

R.C. - ROCK CORE

( ) - PENETROMETER REMARKS:

S.T. - SHELBY TUBE S.S. - SPLIT SPOON

( ) - PENETROMETER

R.C. - ROCK CORE

### MICHIGAN TESTING ENGINEERS, INC.

W = 104LOG OF SOIL BORING NO.

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

PROJECT Hydrogeological Study

406-15046 JOB NO. J

LOCATION Allen Park Clay Mine

Sheet 4 of 4

DATE 8-10-81 Allen Park, Michigan +594.1 SURFACE ELEV.\_ Penetration Moisture Natural Wt. P.C.F. Dry Den Wt. P.C.F. Unc. Comp. Strength PSF. SOIL DESCRIPTION Death Legand 75'6" 76 Fine to medium SAND, gray, saturated (SP). 77 77'0" 15 Fine to medium SAND with a little 8.8 24 78 clay, silt and gravel, gray, moist, 44 very dense (SM)(HARDPAN) 79 79'6" 80 Silty CLAY with a little sand and 50 trace fine gravel, gray, moist, 50/3" 19.0 81 extremely hard (CL) 82 83 84 85 86'0" 86 Boring terminated at 86 Feet 87 Well Data: 88 Monitor Well set at 85'6" 89 below existing grade. 90 Elevation of top of PVC standpipe: +603.82 91 92 93 94 95 96 97 98 99 100 REMARKS: TYPE OF SAMPLE D. - DISTURBED U.L.-UNDIST. LINER



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_

406-15046

PROJECT Hydrogeological Study

W - 105

LOCATION Allen Park Clay Mine

JOB NO. \_ DATE 8-11-81 +594 5

Allen Park Michigan

	SURFAC	E ELEV. +594.5 DATE 8-11-81	Allen Park, Michigan					
Sample Depth	***************************************		netration ws For 6"	Moisture %	Natural Wt. P.C.F.	Dry Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
	0'6"	Silty, clayey SAND, dark brown with						
1	-	organics (Topsoil) (SM-ML)					•	
2		Silty CLAY with a little sand and						
3	MP40SI	trace fine gravel, brown, moist (CL)		KAR-T-C				
3	-		i					
4	7							
5	-							
				***************************************				
6	6'6"	·						
7	0 0	100						
		Silty CLAY with trace sand and fine						
8		gravel, gray, moist (CL)						<del> </del>
9								
			+0-00-00-00-00-00-00-00-00-00-00-00-00-0					
10.	-			29.8				<del> </del>
ST 11		(LL=40, PI=19)						
12	_					-		
12								
13							-	
14	_		-		<u> </u>			
15						<u> </u>		+
16	<u> </u>							
1,3					-			
18								
19			-					+
19								
20				<u> </u>		-		-
ST 21	$\dashv$	(LL=38, PI=19)		33.9				
2		(22 305 11 13)						4
22							<del> </del>	·
23								
								+
24	-							
25								
TVBP OF	e a ve pr	REMARKS:	Ant	rosian	around	Water	   encount	prod
D DIS	Turbed		at	72 6"		a 11461	encount	
S.T SHE S.S SPL	LBY TUBE							
R C - 800	CK CORE IETROMETER	Standard Penetration Test — Oriving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals			Sheet	1 of 4		
( ) - ሦኒስ	EI HUME!EK	140# Hammer Falling 30"; Count Made At 6" Intervals			34,000			

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_

Hydrogeological Study

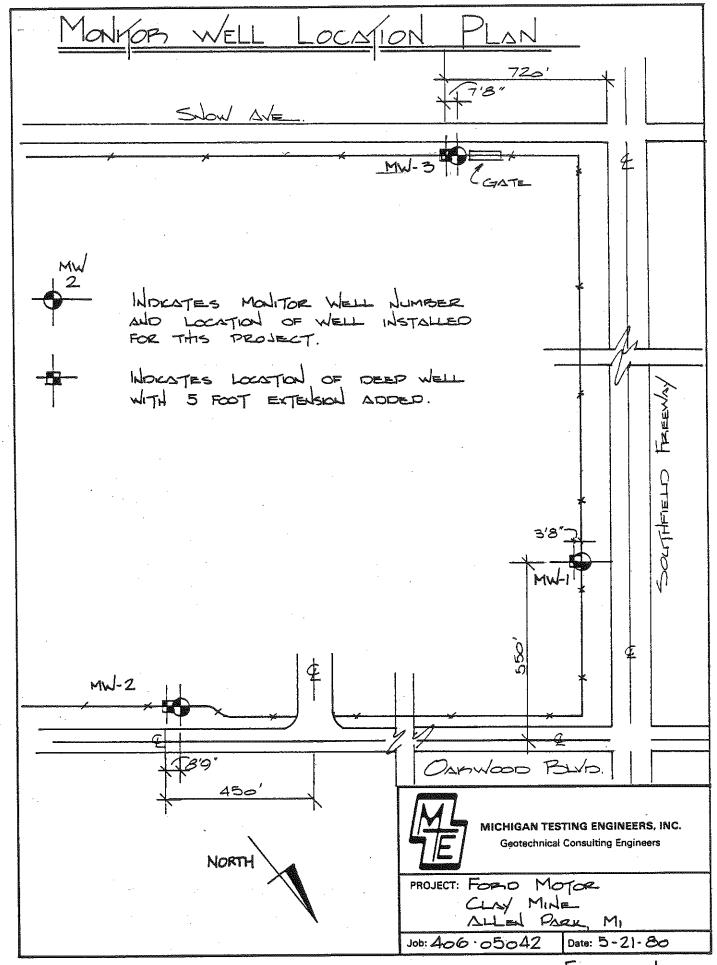
406-15046 JOB NO. \_

Allen Park Clay Mine LOCATION .

+504 5

W-105

OO(11 / NOC CEE1.					Allen Park, Michigan				
Sampia & Type	Cepth	Legend	SQIL DESCRIPTION	Penetration 9lows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Cen Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
	26		OTTER OF BY STOLES TO THE STOLES					-	
	27		Silty CLAY with a little sand and trace fine gravel, moist, gray (C	L)					
	28								_
	29								
	30		( 07		20.1				
ST 3	31		(LL=25, PI=10)		20.1				
	32								
	33						<u> </u>		1.
	34					•			-1
	35								
ST	36		(LL=30, PI=15)		19.7				
4	37								
	38								
	39								
	40				20.4				
ST 5	41		(LL=25, PI=9)		20.4				
	42		•						
	43_		•						
	44		•			-			-
	45								<del>                                     </del>
ST	46		(LL=30, PI=14)		20.8	129.5	107.2		
6	47								
	48								
	49		,						
	50_								
) D.	PE OF SAN	BED	REMARKS:						. ]
S.T	UNDIST. SHELBY 3 SPLIT S	TUBE				•		•	-
R.0	ROCK C ) - PENETF	OMETER	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals		She	et 2 o	f 4	<u>.</u>	



MTE – G1

FIGURE



# MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

W-105 LOG OF SOIL BORING NO. \_\_

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

406-15046 JOB NO. .

		SURFACE	ELEV. +594.5 DATE 8-11-81	DATE 8-11-81			Allen Park, Michigan				
Sample & Type	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Dry Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str.		
	76	8	Medium SAND with a trace fine								
	77	5	gravel, gray, saturated (SP)								
	78										
	79	79'0"						~	POTENTIAL ES		
	80	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Silty SAND and CLAY, hard, slightly	,							
		2' .s	moist (Hardpan)								
	81										
	82	82'6"									
	83		Boring terminated at 82'6"								
	84	5									
	85	n 8 n 9		14 19							
	86		Well Data:								
	87		Monitor well screen set at 82'								
	88		below existing grade.								
	89	27	Elevation of top of PVC standpipe:								
	90		+604.08			5250 N					
	91										
	92	8									
			#								
	93								-		
	94										
	95										
	96										
	97				2)		11		i		
	98	2			To the second			2			
	99	fa 6									
	100										
ŢYI	TYPE OF SAMPLE		REMARKS:								
U.L S.T	- DISTUR -UNDIST. - SHELB	LINER						S 5			
B C	S.S SPLIT SPOON R.C ROCK CORE ( ) - PENETROMETER		Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals	Sheet 4 of 4							

APPENDIX D

MTE SHALLOW WELL INSTALLATION REPORT - JUNE 1980

.



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. .

W-105

PROJECT Hydrogeological Study

LOCATION Allen Park Clay Mine

J0B NO. \_\_\_

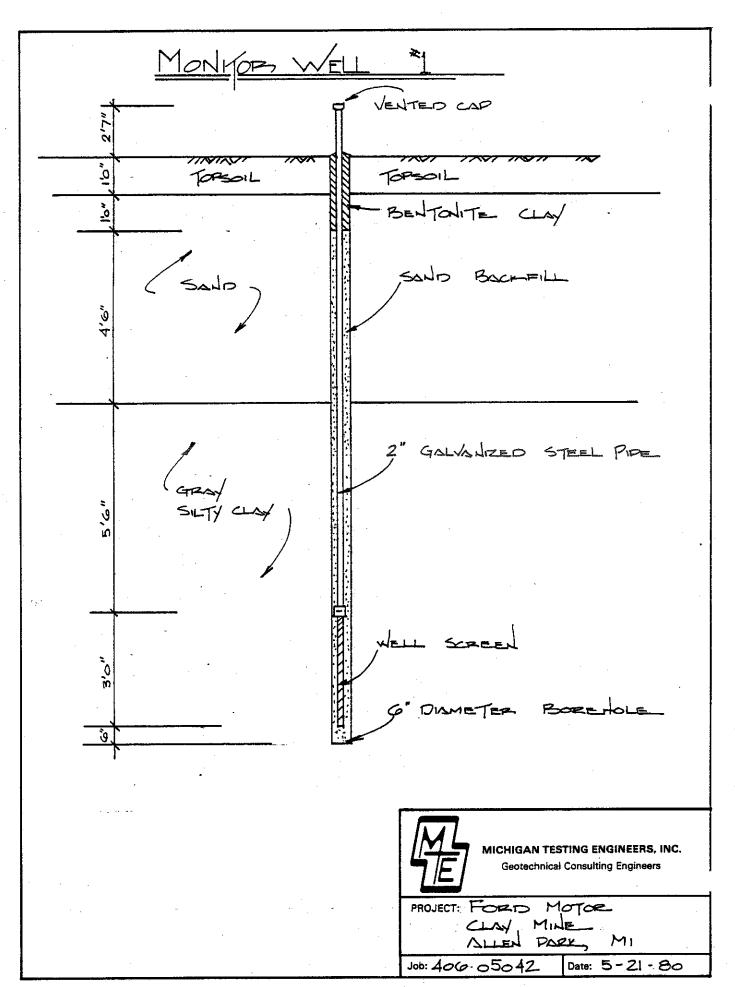
406-15046

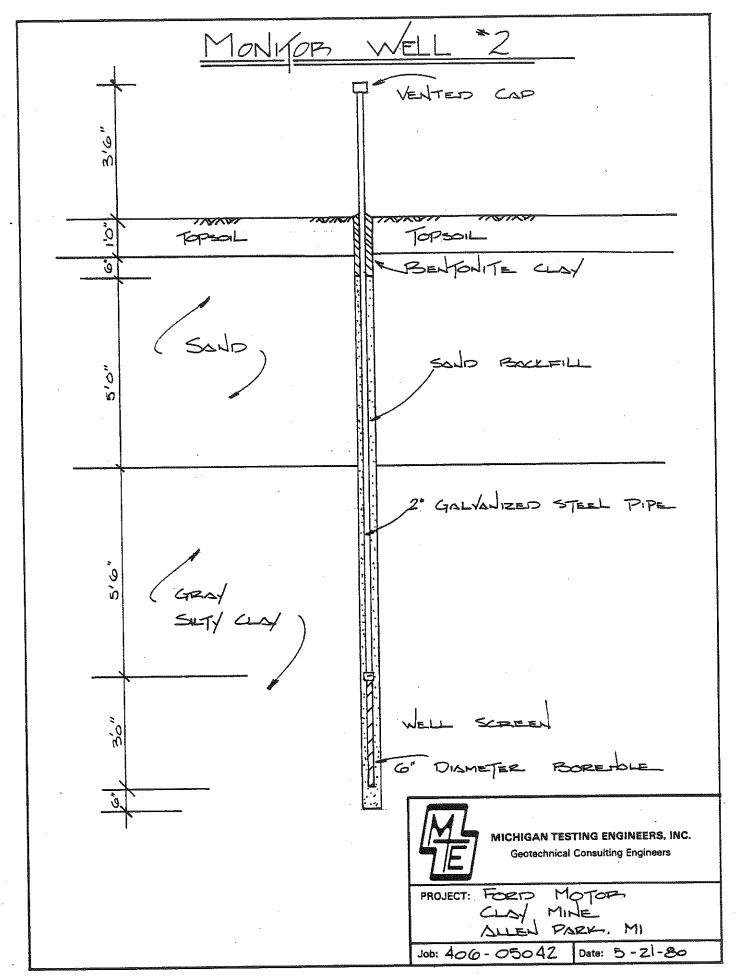
		SURFACE	ELEV +594.5 DATE 8-11-81			Allen Park, Michigan				
Sampia & Type	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6*	Moisture %	Naturai WL P.C.F.	Ory Den Wt P.C.F.	Unc. Comp. Strength PSF.	Str. %	
	51									
	52		Silty CLAY with a little san trace fine gravel, gray, moi	d and st (CL)					· · · · · · · · · · · · · · · · · · ·	
	53									
			_		-	H-2-1-1-2-1-1-1				
	54									
	55		(LL=31, PI=16)		21.5			-		
ST 7	56									
	57									
	58									
	59									
	60									
	61									
	62									
	63									
	64		√ Without gravel							
	65									
ST 8	66		(LL=34, PI=14)		25.6	123.4	98.2			
	67									
	68									
	69								<del> </del>	
	70							-		
	71	]							-	
	72	-								
		72'6"								
	73	1	Medium SAND with a trace fi	ne						
	74	1	gravel, gray, saturated (SP	)						
	75			· ·					<del> </del>	
D. U.L S.T S.S	PE OF SA - DISTUI UNDIST. SHELB SPLIT ROCK PENET	RBED LINER LY TUBE SPOON CORE	REMARKS:  Standard Penetration Test — Driving 2" 0D Samp 140# Hammer Falling 30"; Count Made At 6" in		Shee	t 3 of	4		entropies de la composita de l	

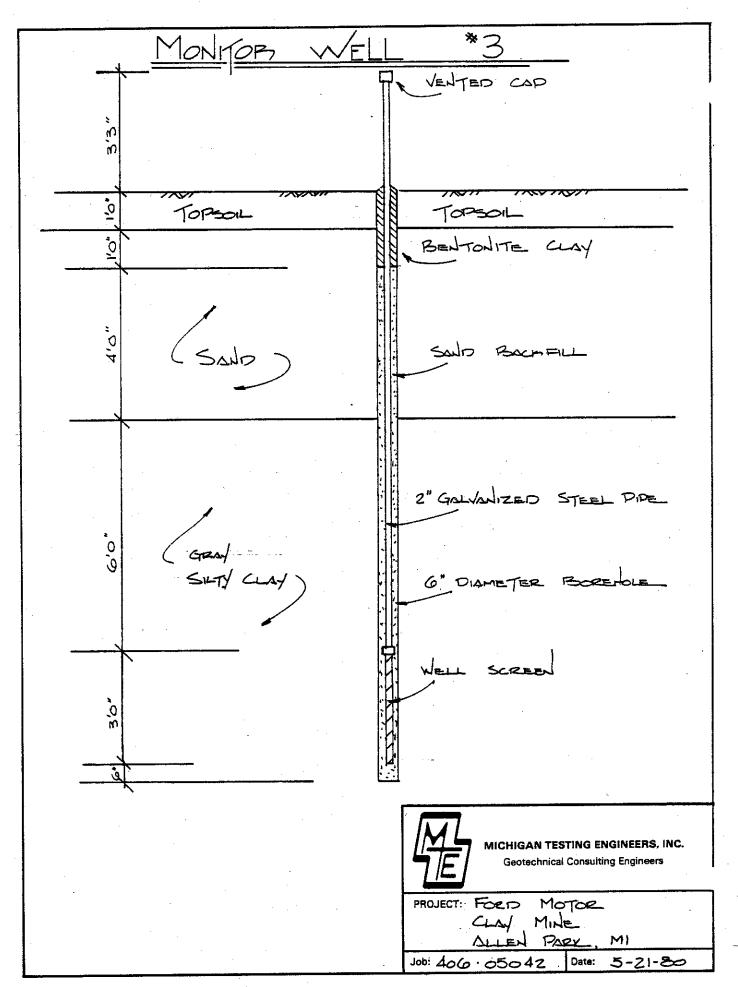
•. . } .

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APPENDIX E

LABORATORY TEST DATA

MTE REPORT - OCTOBER 1979

### LABORATORY TEST RESULTS

Summary of Pertinent Facts From MTE File No. 64-9623

Project:

Allen Park Clay Mine Allen Park, Michigan

Date:

October 1979

Sample No.	Moisture Content, %	Dry Density PCF	Coefficient Of Permeability, cm/sec
Ţ	10.8	126.7	$5.4 \times 10^{-8}$
2	13.2	116.2	9.1 x 10 <sup>-8</sup>

# REPORT OF MOISTURE DENSITY RELATIONSHIP

Visual Classification: Boring:

Silty Clay, with occasional silt seams, blue,

N.A. Depth:

Surface

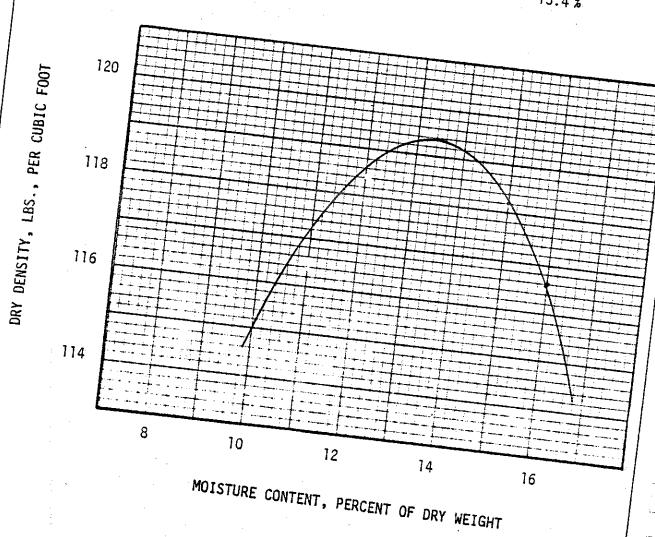
Method of Test: ASTM D 1557-70 (Modified Proctor)

Maximum Dry Density

Optimum Moisture Content

119.4 lbs/ft3

13.4%



### REPORT OF MOISTURE DENSITY RELATIONSHIP

Visual Classification: Silty Clay, with little sand, blue, Moist

Boring: N.A.

Depth: Surface

Method of Test: ASTM D 1557-70 (Modified Proctor)

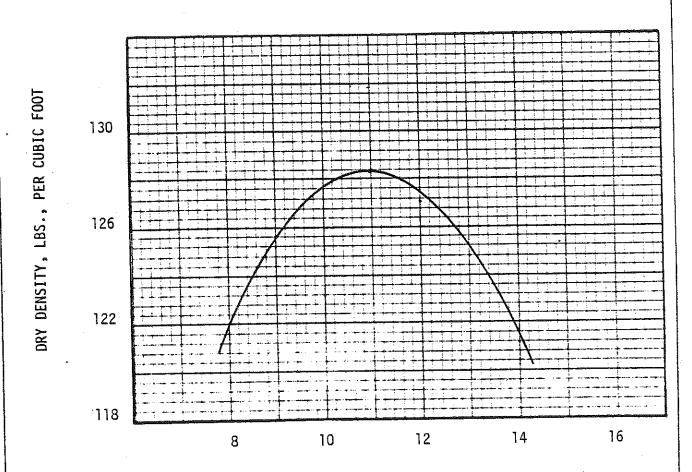
Test Results:

Maximum Dry Density

128.4 lbs/ft<sup>3</sup>

Optimum Moisture Content

11.0 %



MOISTURE CONTENT, PERCENT OF DRY WEIGHT

### LABORATORY TEST RESULTS

Summary of Pertinent Facts from MTE File No. 64-8519

Project: Allen Park Clay Mine

Allen Park, Michigan

Date: March 1978

Boring No.	Depth <u>Feet</u>	Coefficient Of Permeability, cm/sec
3	30	$3.8 \times 10^{-8}$
5	55	$6.0 \times 10^{-8}$
6	5	$6.6 \times 10^{-8}$
8	30	4.5 x 10 <sup>-8</sup>

A water sample was secured from each well on March 8, 1978, and was tested for chemical oxygen demand (C.O.D.), chlorides, iron, sulphates, T.K. nitrogen, pH, phenolics, T.D.S. and carbonate hardness. The results of these tests are listed below.

Test Parameter	Boring #2	Boring #5	Boring #7	Boring #10
C.O.D.	241.0	188.0	150.0	346.0
Chlorides	183.0	165.0	512.0	146.0
Iron	0.01	<0.01	0.06	0.05
Sulfates (SO <sub>4</sub> )	1250.0	300.0	675.0	2150.0
T.K. Nitrogen	2.7	2.5	2.2	3.0
pH .	7.96	8.06	12.03	7.65
Phenolics	22.0	<b>&lt;2.</b> 0	<b>&lt;</b> 2.0	< 2.0
T.D.S.	2389.0	1161.0	2142.0	2812.0
Carbonate Hardness	1300.0	880.0	1050.0	1820.0

All the above test results, except pH and Phenolics are reported as mg/1. Penolics are reported as \_\_ug/1.

These tests were conducted in accordance with <u>Standard Methods for the Examination of Water and Wastewater</u>, 14 Edition.

# APPENDIX F

SOIL BORING AND DEEP MONITORING WELL LOGS

AND LABORATORY TESTING

MTE REPORT - MARCH 1978



# MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_\_ PROJECT Soils Exploration

64-8519 J0B NO. \_\_\_\_\_

LOCATION Allen Park Clay Mine

		SURFACE	ELEV.	593.42 DATE	2-23-78		A1	len Pa	rk, Mi	chigan	····
Sample & Type	Depth	Legend		SOIL DESCRIPTION		Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Cen Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
	4	. / . \ \									
	<u> </u>	\		The same are a second areas	*********						
1A	2	(~~~~)		Firm moist mixed var CLAY, with sand, or		4					
UL	-	\ \ /		streaks and some ve	genic getation	4		<del></del>			
	3	`\ / \		der cared and dome to	500-0	3				/	
		) ××	-	•							
	4	<u>├</u>	3'9"					ļ			
1B						3					
υL	5					3			<b></b>		
	6					-					
		1									
1C	7					3					
UL						4					
	8	`		Firm moist silty bl	ue CLAY	5					ļ
						-					1
1 Ts	9					7			-		
UL UL	10	\				7					<b>†</b>
741		1 \				7					
	11	1									
		$\mathbb{N}$ . $\mid$		•				-			ļ
	12	\					ļ .				ļ
	100	-		,			<u> </u>	<u> </u>	<u> </u>		ļ
	13	1					<u> </u>	<u> </u>	<del>                                     </del>	<del> </del>	
	14	$\setminus$ $\mid$									
1E			14'0"			2					
UL	15	, \		•		3					
		$V$ , $\sqrt{L}$				3					<u> </u>
	16						-	-			ļ
,	17	-							<u> </u>	· · · · ·	
<del></del>	116	1 \	_								<del> </del>
	18	١, ١		Firm moist blue CLA	Y. with						
		]\`.		sand	,						
	19	] \ .									
1F		↓ . \`	,	•		3		1		1	<del> </del>
UL	20	<b>│</b>				3	+		+		-
<u> </u>	21	- '				<del>-</del>			+		<del>                                     </del>
		$\uparrow \setminus$ . $\mid$		•		E					
	22	_ / \		•							
				-							
	23	1 /						-	<u> </u>		
	100	┥. `│							<del> </del>	-	+
	24	4				3		-			<del>                                     </del>
1G	25	$\dashv$ $/$ $'$ $^{-1}$	I		-	4	1				<del>-  </del>
UL		┥, \、				5	1	1			
T	YPE OF SA	MPLE	REMARKS:			1	G	ROUND WA	TER OBSER	VATIONS	
l D	- DISTL L-UNDIST	IRBED					W. ENCOUNT			FT.	INS.
l S.	.t Shel	BY TUBE	]				N. ENCOUNT N. AFTER C			FT. FT.	INS. INS.
l R	.s split .c rock	CORE	Standa	ird Penetration Test — Driving 2" OD S # Hammer Falling 30"; Count Made At	Sampler 1' With	G.1	W. AFTER		HRS.	FT.	INS.
(	) - PENE	TROMETER	140	# Hammer Failing 30"; Count Made At	b' Intervals	G.\	N. VOLUME	3	NONE		



S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE

( ) - PENETROMETER

### MICHIGAN TESTING ENGINEERS, INC.

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. .

PROJECT Soils Exploration

64-8519 JOB NO. \_\_\_\_\_

LOCATION Allen Park Clay Mine

from 0'0" to 55'0"

DATE 2-23-78 Allen Park, Michigan 593.42 SURFACE ELEV.\_\_ Penetration Blows For 6" Unc. Comp Strength PSr. Moisture % Natural Wt. P.C.F. Ory Den WL P.C.F. Sample & Type SOIL DESCRIPTION Depth Legend ŰĹ 4 Firm moist blue CLAY, with υL sand 4 50 55'0" End of Boring 5 6 60 65 70 75 REMARKS: TYPE OF SAMPLE D. - DISTURBED U.L.-UNDIST. LINER Hole filled with Natural Soils

Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

LOG OF "TIL BORING NO. .

MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

PROJECT	Soils	Exploration

LOCATION Allen Park Clay Mine 64-8519 JOB NO. \_\_ Allen Park, Michigan SURFACE ELEV. 595.14 DATE <u>2-27-78</u> Penetration Blows For 6" Moisture % Ory Den Wt. P.C.F. Unc. Comp. Strength PSF. Natural Wt. P.C.F. Str. Sample & Type SOIL DESCRIPTION Depth Legend Firm moist sandy black TOPSOIL 1'6" Compact moist medium brown UL SAND 3 6 316" 4 2B Very compact wet medium brown UL SAND 3 6 6'0" Firm moist silty blue CLAY Ū. 8 9 9'0" 2D 5 ŪL 10 6 11 Stiff moist silty blue CLAY 12 13 14 2E 14'6" 15 υL Firm moist silty blue CLAY 16 17 17'0" 18 19 2F Firm moist silty blue CLAY 3 20 and ROUGE MARKINGS 21 22 23 24 25 UL **GROUND WATER OBSERVATIONS** TYPE OF SAMPLE D. - DISTURBED REMARKS: Tip of Well Point Set 3 G.W. ENCOUNTERED AT 6 INS. @ 77'0" U.L.-UNDIST. LINER G.W. ENCOUNTERED AT INS. 70 FT. 6 S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE 70 HRS. FT. G.W. AFTER COMPLETION 0 INS.

Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

( ) - PENETROMETER

G.W. AFTER G.W. VOLUMES

HEAVY

	& Type Depth	OURFACE ELEV	27.19	Χ,	PRO IDOT
	is gehiu	Legand	593.14		PROJECT_Soil
			SOIL DESCRIPTION	_ DATE_2-27-78	LOCATION Allen P
	2H 30	27'0"			Pener Allan
	显 30 (	\ · }	Firm moist silty and ROUGE MARKING	SS CLAY	Blows For 6" Moisture Matural Wt. P.C.F.
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	H = X	1		}	3 -
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2 <u>1</u> UL	40	1 .	·	4	
		Stif	:e		<del></del>
		sand	f moist blue CLAY, and pebbles		<del></del>
	45		pebbles	with $\frac{2}{4}$	
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2 <u>L</u> 50				2	
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2M 55 UL 55	10.			3	###
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2N 60	58'0"	•.		3	<del></del>
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20 65	, '/	Firm moist	lue CLAY, with	2	
20 65 UL 65		sand and pebb	lue CLAY, With	$\frac{3}{3}$	
	\o_1/				
2P 70	· \/_		ŀ	2	
	69'0"		[3	2	
	70'6" St	ry compact was	dv 1		- <del></del>
20 75	Ve:	Ty compact wet	y blue CLAY	<del></del>	
	74'0"	2 . 465	medium 6	$\pm \pm$	
OF SAME	HARI REMARKS:	PAN and layers	10 10	+	
S.T SHELBY TUBE		yers	of SAND 33		
S.S SPLIT SPOON R.C ROCK CORE ( ) - PENETROMETER			89		
TOTAL TOTAL	Standard Penetratio	n Test _ ~			
in the second	Familier Fa	n Test — Driving 2" OD Sa ling 30", Count Made At 6'	mpler 1' With		
		7.00	IIItervals		1
	•			and the second s	i i i



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

PROJECT Soils Exploration

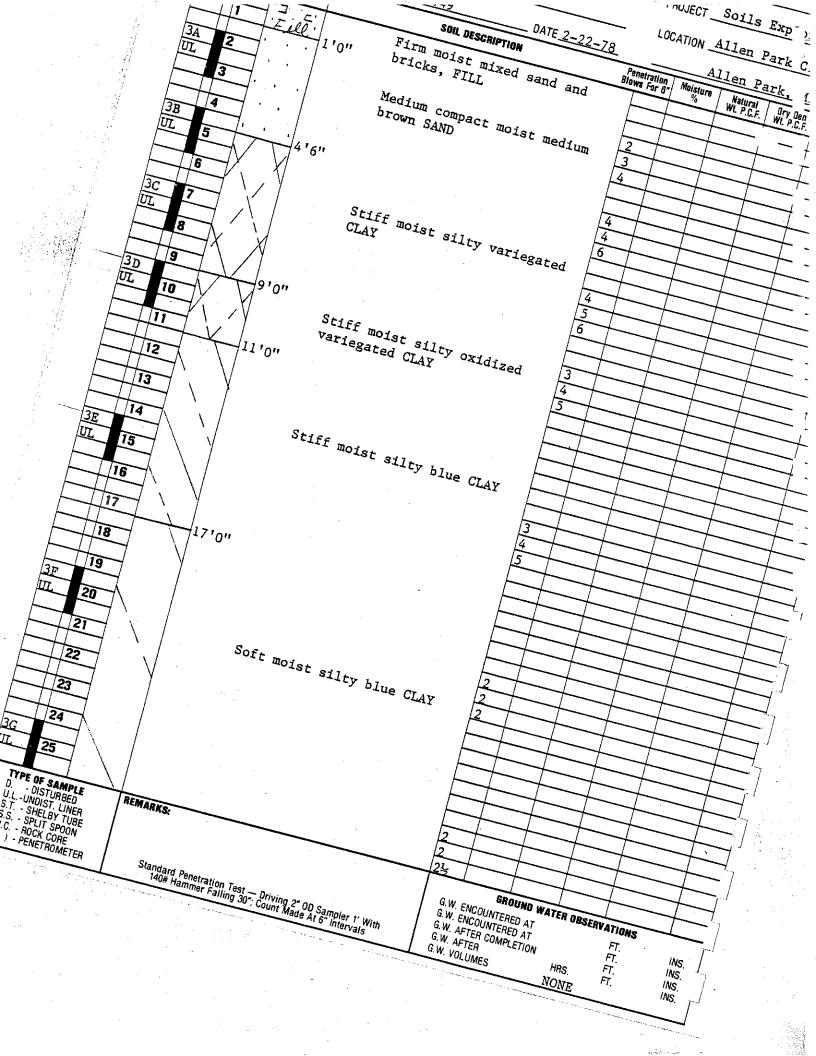
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64-8519 JOB NO. \_\_\_\_

LOCATION Allen Park Clay Mine

Allen Park Michigan

Sample	ï			ELEV. 593.14 DATE 2-27-78	Penetrati	no		Natural			Str. %
Sample & Type	_	Depth	Legend	SOIL DESCRIPTION	Penetrati Blows For	6"	Moisture %	Natural Wt. P.C.F.	Ory Cen Wt. P.C.F.	Unc. Comp. Strength PSF.	%
-	_			HARDPAN and layers of SAND		-					
				77'0" End of Boring							
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Ţ	YF	E OF SA	MPLE	REMARKS:							
U	Ļ.	- DISTU -UNDIST	HBED LINER								
S	.T. .S.	- SHEL! - SPLIT	BY TUBE SPOON		ļ		•			•	
A (	.C.	- ROCK - PENE	IMPLE IRBED TLINER BY TUBE SPOON CORE TROMETER	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals							





CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_\_\_

PROJECT Soils Exploration

JOB NO. \_\_\_\_\_

LOCATION Allen Park Clay Mine

		SURFACE	ELEV598.49	DATE 2-22-/8			en Par			
Sample & Type	Depth	Legend	SOIL DESCRIPTION		Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Cen Wt. P.C.F.	Unc. Comp. Strength PSF.	Str.
7.5-	-						-			
		1	Soft moist silty	blue CLAY						
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		] ,	•							
3J	40	] ' \. [	Firm maint blue	CTAV	2		ļ			
UL			Firm moist blue		3					
		] . " ]	sand and pebbles	3	3	<u> </u>				
		] , ,					-	-		
										<u> </u>
3K	45	] ° ,\·			2			ļ	<u> </u>	
UL		]			3	-				<u> </u>
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					3½	<del>                                     </del>				-
		21	53'0"				ļ	1	<u> </u>	ļ
		<b>1</b> / . ° . / .	Stiff moist blu	e CLAY, with	-					1
3M	55	T, 7 • 7	55'0" sand and pebble	S	2	+		<del> </del>		<del>                                     </del>
ΩĽ		-	End of Boring		5					<del> </del>
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	PE OF SA		REMARKS:	3.0	но	וווים ב	ed v/++	h Natu	ral Soils	2
0.	- DISTU UNDIST	JR8ED		•			to 55		TOT SOITS	•
S.7	r Sheli	BY TUBE			110			J		
l s	S - SPLIT	SPOON	Obnasiani Danasantina Tanka Dalaina	2º OB Camples 4: Islian		٠				
(	) - PENE	CORE TROMETER	Standard Penetration Test — Driving 140# Hammer Falling 30"; Count N	iade At 6" Intervals						



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

PROJECT Soils Exploration

64-8519 JOB NO. \_\_\_\_\_

LOCATION Allen Park Clay Mine

DATE 2-23-78 Allen Park, Michigan 595.07 SURFACE ELEV. \_\_ Penetration Blows For 6" Moisture % Natural Wt. P.C.F. Ory Oen Wt. P.C.F. Unc. Comp. Strength PSF. SOIL DESCRIPTION Depth Legend Firm moist sandy black TOPSOIL 1'0" Medium compact moist medium 3 4A brown SAND 4 UL 3 3 3'0" 4 Firm moist oxidized variegated UL 5 silty CLAY 2 3 6 2 7'0" 2 UL 8 Firm moist silty blue CLAY 3 9 9'0" 2 4D 2 UL 10 2 Soft moist silty blue CLAY 11 12 13 13'0" 14 4E 15 11/2 Soft moist silty blue CLAY, 2 with sand and pebbles, rouge 16 markings 17 18 19 19'0" 2 20 3 21 Firm moist blue CLAY, with sand and pebbles 22 23 24 3 25

TYPE OF SAMPLE - DISTURBED U.L.-UNDIST. LINER

S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE ( ) - PENETROMETER REMARKS:

Standard Penetration Test — Driving 2" 0D Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

G.W. ENCOUNTERED AT G.W. ENCOUNTERED AT G.W. AFTER COMPLETION

G.W. AFTER

G.W. VOLUMES

3

55 FT. FT. HRS.

**GROUND WATER OBSERVATIONS** 

LIGHT

INS. INS.

INS.

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

PROJECT Soils Exploration

J08 NO. \_\_\_\_\_

LOCATION Allen Park Clay Mine

Allen Park, Michigan

		SURFACE ELEV	595.07 DA	TE <u>2-23-78</u>	***************************************	A1	len Par	ck, Mic	higan		-
Sample & Type	Depth	Legend	SOIL DESCRIPTION		Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Oen Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %	
		. \.	-							December 11 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	-
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4H	30	. 0.			2					·	-
UL					3		<u> </u>				4
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											]
4I UL	35				2						4
02			Firm moist blue CL	AY, with	3						1
		, ,	sand and pebbles								
4J	40				1					ļ	$\dashv$
UL.	40				2						1
		. •			3						1
	<del>                                     </del>	√2,									_
4K	45	. \.			2				-		1
UL		. \			3						1
	ļ				3						4
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4L	50		•		2						
UL		5 1			3						4
	- <del> </del>	52 '0''	Stiff moist blue C	TAV and	4						$\dashv$
	,		layers of wet SAND		•						
4M	55	55'0"	End of Boring	•	3					<u> </u>	4
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	200000	MPIF REMARKS:	Washington and the second and the se					<u> </u>	1		_
l D.	<b>Pe of Sa</b> - Distui - Undist	RBED					ed wit		ral Soils	; &	
l ST	UNDIST. SHELB i SPLIT	Y TURE	•		l cem	ent fi	OH U U	EO )	<u>0</u> ر		
9.5 9.0	SPLII	SPOON CORE Star ROMETER Star	ndard Penetration Test — Driving 2" Of 140# Hammer Falling 30"; Count Made J	Sampler 1' With							
<u> </u>	, - PENEI	NUMETER !	40# Hammer Halling 30"; Count Made	At b intervals		***************************************		- -	<u>, 1,555,000,000,000,000,000,000,000,000,00</u>		

LOG	0F	SOIL	BORING	NO.	

Soils Exploration PROJECT \_

64-8519 JOB NO.

LOCATION Allen Park Clay Mine

5

Allen Park DATE 2-25-78 Michigan

		SURFACE	ELEV.	595.68	DATE 2-25-78			Al	len Pa	rk, Mi		
Sample & Type	Depth	Legend		SOIL DESCRIP	TION	Pen 8low	etration s For 6"	Moisture %	Natural Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	tr. %
				Firm moist sa	andy black							
-	1		1'0"	TOPSOIL	· .	1			<del> </del>			1
5A:	2				t medium brown		4					
UL			2 1 3"	SAND	moist medium	ĺ	9					
	3	- //	310"	brown SAND an		}	8		1			
-	4		-		fine brown SAND	ļ						
5B		7 \	4'0"	-			3					
UL	5	X = X			silty variegated	L	4					
	6	/\	n	CLAY	•	-	4		1			
	0	//	6'0"	•		į						
5C	7	/ )			ilty variegated		2					
UL				CLAY			<u>3</u> 3					
	8	` (	8'1"							-		
	9		- <b>-</b>	Til dame.								
5D		Į V	•	rirm moist s:	ilty blue CLAY		2					<u> </u>
UL	10						<u>2</u> 3					<del></del> .
	11	\		•			<u>, , , , , , , , , , , , , , , , , , , </u>			1		
			11'6"									
	12		11'6"	•								
	13								1			
-	13								1			
	14				•							
5E			-		•		2			ļ		
UL	15						2					
	16			· - · · · ·	•							
			•									
	17										<u> </u>	<u> </u>
	18			Soft moist s	ilty blue CLAY					-		
	1.5	1			•							
	19	]			•							
5F	20	<u> </u>					2	<u> </u>	1			
UL.	20						2					
	21	1 \										
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	23	1		•								
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	24					•						
5G	25	<u> </u>					1	<u> </u>	-		1	<u> </u>
UL		\		•		_	2					
	E OF SA		REMARKS:	Tip of Well F	oint Set				ROUND WAT			
U.L	DISTUI- UNDIST.	LINER		@ 79'0"				. ENCOUNT . ENCOUNT		3 73	гт. 0 гт. 0	INS. INS.
I S.S	- SHELB - SPLIT	SPOON					G.W	. AFTER CO		2.5 HRS.	FT. O	INS.
R.C	ROCK - PENET	CORE ROMETER	Standa 140	ard Penetration Test — Dri # Hammer Falling 30": Cou	ving 2" OD Sampler 1' With int Made At 6" Intervals			r. AFTER I. VOLUMES	3	HEAVY	гі.	INO.
		-	, 10									

Ł	ΩG	OF	SOIL	BORING	NO.	

PROJECT Soils Exploration

FNOJECI

LOCATION Allen Park Clay Mine

JOB NO. 64-8519

		SURFACE	. ELEV	595.68 DATE 2-25-78					rk, Mi	<del></del>	
Sample & Type	Depth	Legend		SOIL DESCRIPTION	Penetratio Blows For	n Moi 6"	sture %	Natural Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
				Soft moist silty blue CLAY							
				Soft moist sifty bide chai							
			29'0"								
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5I	35				3		<del></del>				
UL.		· ·		Firm moist blue CLAY, with	3						
				sand and pebbles		•				·	
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5J UL	40				3						-
بار					3!	2					
		c	•			_					
577	45	/\ \ \			2	_					+
5K UL	40	' \			3						
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5L	50		49 ' 0''		3		·		<u> </u>		
T.	50	6			4						
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5N	60	, ,		Stiff moist blue CLAY, with	3						
UL		,		sand and pebbles	4	_					
					4	-					+-
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50	65				3						
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5₽ 	70				3						-
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		'5.	721011								丁
			73'0"	Compact wet medium gray SAN					<u> </u>		$\perp$
5Q UL	75	` ` `			D 4	_			<u> </u>	<del>- </del>	+
	E OF SAI	WPLE .	REMARKS:		8		umderstande (Vallet)	1	<u> </u>		
D.	- DISTUF - UNDIST. - SHELB	IBED LINER									
S.T.	- SHELB	Y TUBE SPOON									
. ت. ت	- ROCK (	2000	1	ard Penetration Test — Driving 2" OD Sampler 1' With # Hammer Folling 30"; Count Made At 6" Intervals	- 1						



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG	OF SOIL	BORING NO	)	5	 
				-	

LOCATION Allen Park Clay Mine

PROJECT \_

64-8519 J08 NO.\_

Soils Exploration

Allen Park, Michigan 595 68 DATE 2-25-78

		SURFAC	ELEV595.	68 DATE 2-2				len Pa:			
noie ype	Cepth	Legend		SOIL DESCRIPTION	8	Penetration lows For 6"	Maisture %	Natural Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	St %
		. , ,	Сотр	act wet medium gray	z SAND						
		1 ,	77 '0"								ļ
$\bot$			Extr	emely compact wet m	nedium			* * * **			<u> </u>
			gray	SAND		22					
	80	<u> </u>	80'0" End	of Boring		20					
						39					
$\dashv$			· · · · · · · ·			55					-
-++	85										
$\dashv$	65		•	•							
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		1	REMARKS:				L	·		<u> </u>	

S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE ( ) - PENETROMETER

Standard Penetration Test — Driving 2" 00 Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

	Marian Company
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CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG	0F	SOIL	BORING	NO.	6	

Soils Exploration PROJECT \_

JOB NO. 64-8519

LOCATION Allen Park Clay Mine

		SURFACE	ELEV.	595.03	DATE3 <u>_2_78</u> _		A.	Llen Pa	ırk, Mi	chigan	
Sample & Type	Depth	Legend		SOIL DESCRIPTION	) N	Penetration Blows For 6"	Moisture %	Naturai Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
				Sandy TOPSOIL	-						
	1		1'0"	sandy forsoff							
		M. M	1 0				***				
6A	2	N&VI				6					
UL		/ \./ .\				7					
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		∤. / \				-					
75	4	$\mathbb{N}$				<u> </u>					
6B	<u> </u>	1X . AL				3	***************************************				
UL	5	{				<u>5</u> 5					
	6			0.156							
	-	1 1/1 1		Stiff moist va		•					
6C.	7	M		with sand and	silt, some	6			*		
DL.	<b>-</b>	1/1 <b>1</b> /\		vegetation		7					
- 11	8	1 1 1				7					
-		-									
	9								<del> </del>		
6D ·	3	$\mathbb{N}^{-1} \setminus \mathbb{I}$				3					
UL	10	₹\\\/\				4					
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	11	$\uparrow \land \downarrow$				**********					
		1./ - \									
	12	1/ . N									
		(%)									
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		1	13'0"		•						
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UL.	15	] , , ,	٠			2		<u> </u>		ļ	
						2					
	16						<u> </u>				
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		1 ' / .						-			+
	18	-		Soft moist blu	e'CLAY with				+		
	40	4.		sand	ie Char, with		<del> </del>	· · · · · · · · · · · · · · · · · · ·	ļ	+	
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6F	20					2	-		<del></del>		+
UL	- ZU	4.				2	<u> </u>	<del> </del>	1	+	
	21	-					<u> </u>	<del> </del>	1		1
-	1 21	-			*		1		<del>                                     </del>	<del>                                     </del>	1
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	11	<del> </del> ,									
	24	-			•		1				
6G		<b>-</b> ,				2					
UL	25	1				2					
		1, \				2					
T	PE OF SA	AMPLE	REMARKS:	**************************************	<u>, 144,04,444</u>		G	ROUND WA	TER OBSER	RATIONS	
j D.	- DISTL	JRBED					W. ENCOUNT			FT.	INS.
l S.	LUNDIST T SHEL	By Tube					W. ENCOUNT			FT. FT.	INS. INS.
S.	S SPLIT C ROCK	SPOON	0	and Depotration Test Detail	na 2" 00 Samples 1' \\$645		W. AFTER G W. AFTER	OMPLETION	HRS.	FI.	INS.
(	) - PENE	TROMETER	Standa 140	ard Penetration Test — Drivi # Hammer Falling 30"; Coun	t Made At 6" Inter : als		N. VOLUME	s ·	NONE		

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( ) - PENETROMETER

#### MICHIGAN TESTING ENGINEERS, INC.

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_\_\_

Soils Exploration

64-8519 JOB NO. \_\_\_\_\_

LOCATION Allen Park Clay Mine

595.03 DATE 3-2-78 Allen Park, Michigan SURFACE ELEV.\_ Unc. Comp. Strength PSF. Str % Penetration Blows For 6" Moisture % Natural Wt. P.C.F. Dry Den Wt. P.C.F. Sample & Type SOIL DESCRIPTION Depth Legend Soft moist blue CLAY, with sand 29'0" 3 **6H** 30 UL 3 The Business 4 **6**I 35 UL 4 Firm moist blue CLAY, with 4 3 **6**J 40 UL 3 3 6K 45 3 UL 4 49 '0" 3 6L 50 Stiff moist blue CLAY, with 4 UL sand and small stones 5 6M 55 3 55'0" End of Boring UL 4 60 TYPE OF SAMPLE
D. - DISTURBED
U.L-UNDIST. LINER
S.T. - SHELBY TUBE
S.S. - SPLIT SPOON
R.C. - ROCK CORE REMARKS: Hole filled with Natural Soils from 0'0" to 55'0" Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

REMARKS:

Tip of Well Point Set

Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

@ 79'0"

TYPE OF SAMPLE D. DISTURBED

U.L.-UNDIST. LINER S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE

( ) - PENETROMETER

1.0G	OF	SOIL	BORING	NO.	

PROJECT Soils Exploration

GROUND WATER OBSERVATIONS

G.W. ENCOUNTERED AT

G.W. ENCOUNTERED AT

G.W. AFTER

G.W. VOLUMES

G.W. AFTER COMPLETION

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7/Z

72 HRS.

MEDIUM-HEAVY

FT.

FT.

FT.

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INS.

INS.

INS.

INS.

LOCATION Allen Park Clay Mine 64-8519 JOB NO. \_\_\_ 594.11 Allen Park, Michigan DATE 2-28-78 SURFACE ELEV. Penetration Blows For 6" Unc. Comp. Strength PSF. Sample & Type Moisture Natural WL P.C.F. Ory Den Wt. P.C.F. SOIL DESCRIPTION Depth Legend Firm moist sandy black 0'6" TOPSOIL 7A Firm moist silty variegated UL 3 CLAY 3 3 4 2 7B 2 5 6 6'0" 7C UL. Soft moist silty variegated 2 8 CLAY 9 3 7D 3 UL 10 4. 10 4 4 11 11 12 Firm moist silty blue CLAY 13 14 UL 15 16 17 17'0" 18 19 7F 20 Soft moist blue CLAY, with TIT sand and pebbles, rouge 21 markings 22 23 24 7G 25 UL



# MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG	OF SOIL	BORING	NO		7		•	
	P	ROJECT _	Soils	Explora	tion	<u> </u>		

64-8519 JOB NO. \_

LOCATION Allen Park Clay Mine

			E ELEV DATE 2-28-78			Allen Park, Michigan						
Sample & Type	Depth	Legend		SOIL DESCRIPTIO	N .	Per Biov	netration vs For 6"	Moisture %	Natural Wt. P.C.F.	Dry Gen Wt. P.C.F.	Unc. Comp. Strength PSF.	Str %
		/ '		Soft moist blue	CLAY with							
		· '		sand and pebble								
		1, ' ' ' '		markings	J, 10050							
		/° , '	29 '0''									<u> </u>
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7I UL	35			Firm moist blue	CLAY, with		2					
UL P		^		sand and pebble			3					ļ
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7J	40	ļ · · ·					2					
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UL		١.					3				•	
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7v	45	\ '	]				2					
7K UL	40		ļ		•		2					
<u> </u>			46'0"		•		3					
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77	50						3					***********
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7M UL		10 1.					4					
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			58'0"									
7N	60	· 6				,	2					
UL	-			•			3					
							3					
<del></del>				Firm moist blue				,				
				sand and pebble	S	- 1						
70	65	ς •				ı	2					
UL				• •		l	2					
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		]. ເ	69'0"			[		•		·		
7P	70		69.0	Stiff moist blue	- CTAV		3	_				
UL.	,	1. 0.		sand and pebble			4		,			
		``.c	72 '0"	saud and bennie	<b>5</b>	[	5					
		4 , 6	/2 0	Compact wet med:	ium eras CAND	[						
			74'0"	_	- •							
7Q	75	0 1/11	, , ,	Extremely stiff			18					
υL		1 1. 101		CLAY, with sand	and pebbles	[	21					
TYP	E OF SA		REMARKS:				24					
	- DISTUR Undist.			•		ľ						
S.T.	- SHELB	Y TUBE		•		- 1						
	- SPLIT S		C42-	rd Depatration Test - Debuis-	7" Of Camples 4" With	1						
( )	- PENETI	ROMETER	140	rd Penetration Test — Driving 2 # Hammer Failing 30"; Count Ma	ade At 6" intervals							



# MICHIGAN TESTING ENGINEERS, INC. CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

.OG	OF	SOIL	<b>BORING</b>	NO.	

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64-8519

LOCATION Allen Park Clay Mine

JOB NO. \_\_\_\_\_ 594.11 Allen Park, Michigan DATE 2-28-78 SURFACE ELEV.\_\_\_

Sample & Type	1	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %
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D. - DISTURBED
U.L.-UNDIST. LINER
S.T. - SHELBY TUBE
S.S. - SPLIT SPOON
R.C. - ROCK CORE
( ) - PENETROMETER

Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

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( ) - PENETROMETER

#### MICHIGAN TESTING ENGINEERS, INC.

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

DG	۵F	SOIL	BORING	NO.	

G.W. VOLUMES

NONE

	0-41-	T 1	
PROJECT _	20112	Explo	ration

JOB NO. \_\_\_\_

64-8519

LOCATION Allen Park Clay Mine

DATE 2-28-78 593.91 Allen Park, Michigan SURFACE ELEV.\_ Penetration Blows For 6" Unc. Comp. Strength PSF. Moisture Natural Wt. P.C.F. Ory Gen Wt. P.C.F. Sample & Type SOIL DESCRIPTION Legend Depth TOPSOIL 016" 8 2 **8A** Firm moist variegated CLAY, 3 UL 90 with sand and vegetation 4 3 4 3 8B 5 3 UL 4 5'6" 6 8C 4 UL 8 5 Firm moist brown CLAY, with 9 sand 4 8D 10 5 UL 5 11 12 13 13.0" 14 8E 15 16 17 18 Soft moist blue CLAY, with 19 sand and pebbles 8F 20 .0 21 0 22 23 24 8G 3 25 3 REMARKS: GROUND WATER OBSERVATIONS TYPE OF SAMPLE D. - DISTURBED U.L.-UNDIST. LINER G.W. ENCOUNTERED AT INS. G.W. ENCOUNTERED AT FT. INS. S.T. - SHELBY TUBE S.S. - SPLIT SPOON G.W. AFTER COMPLETION FT. INS. HRS. R.C. - ROCK CORE FT. Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals G.W. AFTER INS.



CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_\_\_

PROJECT Soils Exploration

64-8519 JOB NO. \_\_\_\_\_

LOCATION Allen Park Clay Mine

593.91 DATE 2-28-78

Allen Park, Michigan

Sample			ELEV. J9J.91 UAIE Z 20 70	Penetration	Moisture	Natural	Dry Cen	Unc. Como.	Str.
Sample & Type	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Den Wi. P.C.F.	Unc. Comp. Strength PSF.	Str. %
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UL		1 \		3					
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		1	sand and pebbles						
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UL				3					
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U.1	LUNDIST.	. LINER				' to 55			
1 S.S	T SHELE S SPLIT	SPOON							
T B.	C ROCK	CORE TROMETER	Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals						
{	; - PENE	HUNE I EK	140# Hammer Hailing 30"; Count Made At 6" Intervals						



( ) - PENETROMETER

## MICHIGAN TESTING ENGINEERS, INC.

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

PROJECT Soils Exploration

64-8519 JOB NO. \_\_\_\_

LOCATION Allen Park Clay Mine

\_ DATE 3-1-78 Allen Park, Michigan 593.73 SURFACE ELEV.\_\_\_ Unc. Comp. Strength PSF. Penetration Blows For 6" Moisture % Natural Wt. P.C.F. Dry Gen Wt. P.C.F. Sampie & Type SOIL DESCRIPTION Depth Legend Sandy TOPSOIL 01911 9A 2 Very compact moist brown SAND, UL 8 with some vegetation 11 3 3'6" 4 8 9B 5 8 Very compact wet brown SAND 8 6 6 9C ÜL 7 716" 8 8 9 9D 6 8 ūL 10 10 11 Stiff moist blue CLAY, with sand and streaks of sand 12 13 14 9E 15 UL 16 17 17'0" 18 19 9F 20 UL Soft moist blue CLAY, with sand 21 22 23 24 9G UĽ 25 REMARKS: GROUND WATER OBSERVATIONS TYPE OF SAMPLE - DISTURBED INS. G.W. ENCOUNTERED AT U.L.-UNDIST. LINER G.W. ENCOUNTERED AT INS. S.T. - SHELBY TUBE G.W. AFTER COMPLETION FT. INS. S.S. - SPLIT SPOON R.C. - ROCK CORE HRS. FT. INS. G.W. AFTER Standard Penetration Test — Driving 2" 00 Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

G.W. VOLUMES

NONE

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CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO.

PROJECT Soils Exploration

64-8519

LOCATION Allen Park Clay Mine

JOB NO. \_\_\_\_ 593.73 DATE 3-1-78

Allen Park, Michigan

		SURFACE	ELEV. 593.73 DATE 3-1-78		Allen Park, Michigan						
Sample & Type	Depth	Legend	SOIL DESCRIPTION	Penetration Blows For 6"	Moisture %	Natural Wt. P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str. %		
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D.	E OF SAN - DISTUR	BFO :	REMARKS:	Ho1	e fil	led wit	h Natu	ral Soils	3		
U.L	UNDIST.	LINER				' to 55					
S.S.	- SHELBY	SPOON									
R.C.	- ROCK C	ORE	Standard Penetration Test — Driving 2" 0D Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" intervals	ĺ							
( )	- PENET	NUMETEK	140# Hammer Falling 30"; Count Made At 6" intervals								

S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE ( ) - PENETROMETER

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Lua	ur	SOIL	DUNINU	MO.	-

Soils Exploration PROJECT \_\_

64-8519

LOCATION Allen Park Clay Mine

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FT.

FT.

HRS.

HEAVY

G.W. ENCOUNTERED AT

G.W. AFTER

G.W. VOLUMES

G.W. AFTER COMPLETION

INS.

INS.

INS.

JOB NO. \_\_\_ 593.40 DATE 3-2-78 Allen Park, Michigan SURFACE ELEV. Penetration Blows For 6" Unc. Comp. Strength PSF. Natural Wt. P.C.F. Ory Oen Wt. P.C.F. Moisture SOIL DESCRIPTION Depth Legend Firm moist sandy black TOPSOIL 1'0" Compact moist medium brown 3 2 10A 2 10" 4 UL 5 3 4 Compact wet medium brown SAND 3 10B 5 4 UL 4 6 2 10C 7'0" 3 UL 3 Firm moist blue CLAY and 8 SILT 9 9'0" 3 10D 10 4 UL 5 11 12 Stiff moist blue CLAY and 13 SILT 14 10E 15 UL 6 16 17 18 18'0" 19 10F 20 Ш Firm moist blue CLAY, SILT 21 and ROUGE MARKINGS 22 23 24 10G UL GROUND WATER OBSERVATIONS REMARKS: TYPE OF SAMPLE Tip of Well Point Set D. - DISTURBED U.L.-UNDIST. LINER INS. G.W. ENCOUNTERED AT 060 @ 81'0"

Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

ML

( ) - PENETROMETER

# MICHIGAN TESTING ENGINEERS, INC.

CONSULTING ENGINEERS IN SOILS & FOUNDATIONS

LOG OF SOIL BORING NO. \_\_

PROJECT Soils Exploration

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LOCATION Allen Park Clay Mine

JOB NO. \_\_\_\_\_ Allen Park, Michigan SURFACE ELEV. 595.40 \_ DATE\_<u>3-2-78</u>\_ Penetration Blows For 6" Moisture % Natural Wt. P.C.F. Dry Gen Wt. P.C.F. SOIL DESCRIPTION Depth Legend Firm moist blue CLAY, SILT, and ROUGE MARKINGS 29'0" 30 10H Stiff moist blue CLAY, with UL 4 sand and pebbles 34 1011 2 3 UL 3 10J Firm moist blue CLAY, with 3 sand and pebbles 2 10K 45 3 3 2 10L 50 3 51'0" 4 10M Stiff moist blue CLAY, with sand and pebbles 60 10N 5  $\Pi L$ 0 6 63'0" 65 100 UL Firm moist blue CLAY, with sand and pebbles 10P 70 3 10Q Very compact wet medium 7516" 3 UL brown SAND REMARKS: TYPE OF SAMPLE D. - DISTURBED U.L.-UNDIST. LINER S.T. - SHELBY TUBE S.S. - SPLIT SPOON R.C. - ROCK CORE Standard Penetration Test — Driving 2" OD Sampler 1' With 140# Hammer Falling 30"; Count Made At 6" Intervals

Image: Control of the control of the	MICHIGAN TESTING ENGINEERS, INC.	LOG OF SOI
	CONSULTING ENGINEERS IN SOILS & FOUNDATIONS	

JOB NO.

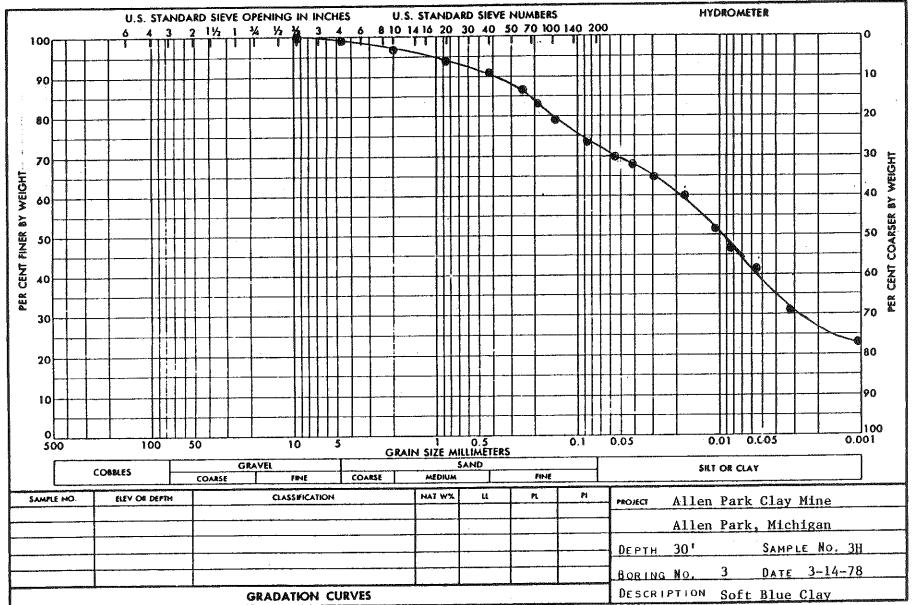
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Soils Exploration PROJECT \_\_

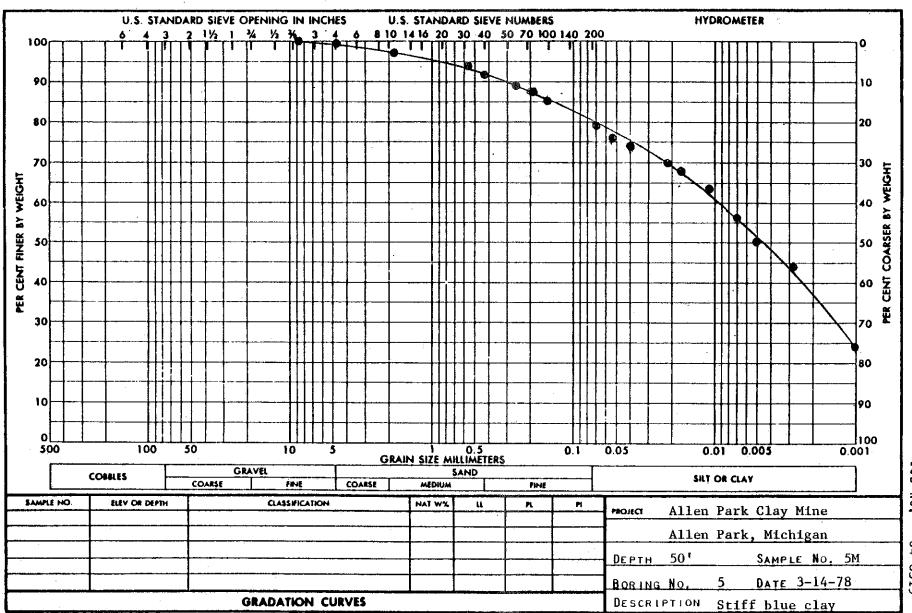
LOCATION Allen Park Clay Mine 64-8519

Allen Park Michigan

	SURFACE ELEV		595.40								
Sample & Type	Depth	Legend		SOIL DESCRIPTI	ON	Penetration Blows For 6"	Meisture %	Natural WL P.C.F.	Ory Den Wt. P.C.F.	Unc. Comp. Strength PSF.	Str %
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				Very compact w brown SAND	et medium						
		, , ,	79 <b>''</b> 0''								
	80		19 0	Silty HARDPAN		17 25					
UL 📮		=	81'0"	End of Boring		62					
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U.LUI	NOIST.	LINER	,	,						·	
S.S : R.C	SPLIT S BOCK C	POON ORE	Standa	rd Penetration Test — Drivin	g 2" OD Sampler 1' With						`
( ) -	PENETF	OMETER	140	rd Penetration Test — Drivin # Hammer Falling 30"; Count	Made At 6" Intervals						

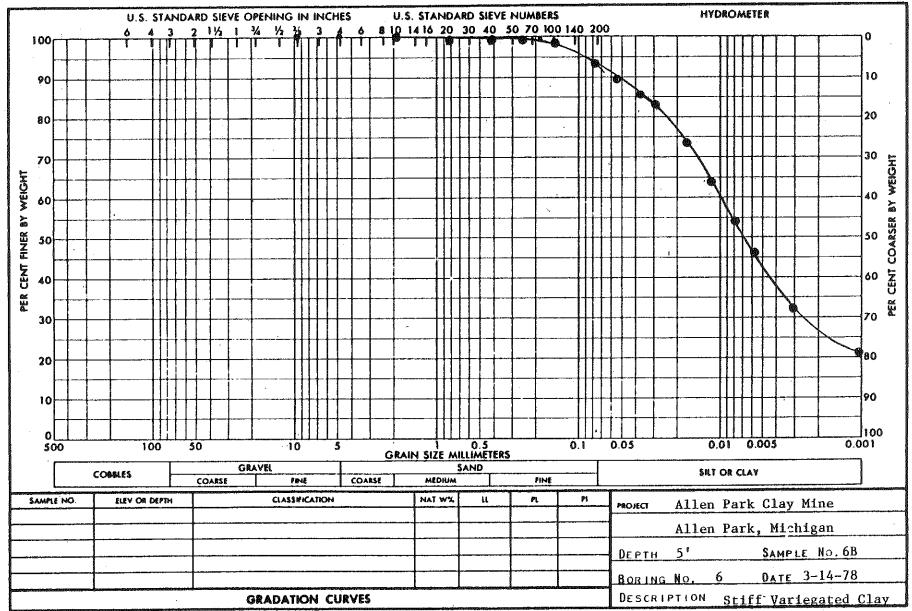


JOB NO. 64-8519



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# APPENDIX G

SOIL BORING FROM VETERANS HOSPITAL LOCATED AT OUTER DRIVE AND M-39